REPORT OF ADDITIONAL INVESTIGATION

Port of Tacoma Former Milwaukee Railyard Tacoma, Washington

Shaw Project No. 111487

September 2008

Submitted to:

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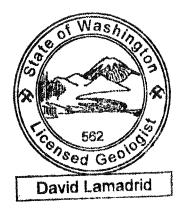
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The material and data in this report were prepared under the supervision and direction of the undersigned.

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Executive Summary

Additional investigation activities were performed at the former Milwaukee Railyard Site (Site) in Tacoma, Washington, to evaluate the progress of the cleanup actions and compliance with the groundwater cleanup standards established for the Site as part of a Prospective Purchaser Consent Decree (PPCD). The Port of Tacoma entered into the PPCD with the Washington Department of Ecology (Ecology) in 1994 to conduct cleanup actions at the Site associated with historical releases of petroleum and to resolve the potential liability for petroleum hydrocarbons in the soil and groundwater at the Site.

The historical releases of petroleum hydrocarbons were associated with former railyard operations prior to acquisition of the Site by the Port of Tacoma. Site investigation activities performed in the early 1990s identified concentrations of total petroleum hydrocarbons in soil and groundwater and the presence of nonaqueous phase liquids (NAPL). Site demolition activities have removed all potential sources of NAPL from the Site. The hydrocarbons are comprised predominantly of highly weathered diesel fuel with lesser amounts of heavier lube or fuel oils, located in the east-central portion of the Site. The investigations also showed that the NAPL was not migrating to the nearby Milwaukee Waterway (which was subsequently filled, in 1995) or the Sitcum Waterway. Following the initial investigations, a remediation system consisting of 22 NAPL recovery wells was installed during 1997 in accordance with the PPCD, and a total of approximately 63,700 gallons of NAPL has been recovered through 2007. Since 2004, the recovery rate and volume recovered have declined, and the system produces mostly water.

The additional investigation, as approved by Ecology pursuant to a prepared work plan, included three primary tasks:

- Task 1 consisted of groundwater sampling to measure the concentrations of petroleum hydrocarbons and related constituents for comparison to groundwater cleanup standards listed in the PPCD.
- Task 2 consisted of collecting undisturbed soil core samples within the NAPL zone for direct observation and testing of physical properties (e.g., grain-size distribution, porosity, permeability, air/water/NAPL content).
- Task 3 consisted of collecting NAPL samples for chemical and physical characterization.

The results of the additional investigation demonstrate that the remaining residual NAPL is not impacting groundwater at levels above the established cleanup levels. Additionally, potentially hazardous constituents including benzene, toluene, ethyl benzene, and total xylenes; polynuclear

aromatic hydrocarbons; naphthalene; and polychlorinated biphenyls were not detected in offsite compliance wells MW-21, MW-22, and MW-23, which are located downgradient or crossgradient (at the point of compliance) from the area where NAPL was previously documented. In addition, measurable NAPL accumulations are now present in only two wells (CW-4 and CW-5) since September 2005.

Gas chromatography (GC) testing of NAPL samples collected from select recovery wells and monitoring wells show that residual NAPL at the Site has undergone extensive biological, chemical, and/or physical alteration resulting from exposure to the natural environment (i.e., weathering). The GC analyses indicate that the remaining NAPL is highly weathered diesel and heavier fuel oils that are immobile under non-pumping conditions and have been depleted of water-soluble hazardous substances. Additionally, the results of soil coring observations and testing demonstrate that the residual NAPL at the Site is predominantly restricted to finer-grained soils, which are less amenable to product removal, and that pore fluids are primarily saturated with water with comparatively small amounts of NAPL (2.9 percent to 8.8 percent). The results also show that NAPL is not extractable from the soil-pore throats at the test boring SC-1 location, but is extractable from the soil-pore throats at the test boring SC-2 location.

The remediation system produces mostly water from the coarser-grained soils when it is operated. Recovery data show that the remediation system recovers less than 0.3 percent NAPL by volume.

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Acronyms and Abbreviations_____

AGI Applied Geotechnical, Inc. API American Petroleum Institute

bgs below ground surface

BTEX benzene, toluene, ethyl benzene, and total xylenes

cP centipoise

Ecology Washington Department of Ecology

GC gas chromatography
gm/ml gram(s) per milliliter
mg/kg milligram(s) per kilogram
mg/L milligram(s) per liter
NAPL nonaqueous phase liquid

PAH polynuclear aromatic hydrocarbon

PCB polychlorinated biphenyls

PPCD prospective purchaser consent decree

Shaw Shaw Environmental, Inc.
Site former Milwaukee Railyard site
TechSolv TechSolv Consulting Group, Inc.
TGI Torkelson Geochemistry, Inc.
TPH total petroleum hydrocarbon

UPRR Union Pacific Railroad

USPCI United States Pollution Control Inc.

UST underground storage tank
Work Plan Additional Studies Work Plan

1.0 Introduction

This report presents the findings and conclusions of additional investigation activities performed at the former Milwaukee Railyard Site in Tacoma, Washington (Site, Figure 1). The Port of Tacoma entered into a Prospective Purchaser Consent Decree (PPCD) with the Washington Department of Ecology (Ecology) in 1994 and conducted cleanup actions at the Site associated with historical releases of petroleum. The objective of this investigation is to evaluate the progress of the cleanup actions and compliance with the groundwater cleanup standards at the points of compliance identified in the PPCD and the associated Cleanup Action Plan.

The objectives and proposed tasks for this investigation were discussed during an August 30, 2007 meeting with the Port of Tacoma, TechSolv Consulting Group, Inc. (TechSolv), Shaw Environmental, Inc. (Shaw), and Ecology. The technical approach was described in a letter from TechSolv to Ecology dated September 9, 2007 (TechSolv, 2007a). TechSolv subsequently prepared an Additional Studies Work Plan (Work Plan), dated October 15, 2007 (TechSolv, 2007b). The Work Plan was approved by Ecology in a comment letter dated October 31, 2007 (Ecology, 2007). The Work Plan and this report were implemented and prepared by Shaw and TechSolv.

1.1 Prospective Purchaser Consent Decree

The purpose of the PPCD is to resolve the potential liability for petroleum hydrocarbons in the soil and groundwater at the Site. The PPCD is also intended to promote the public interest by expediting cleanup activities and site redevelopment.

Cleanup standards for soil and groundwater are identified in the PPCD. The cleanup level for total petroleum hydrocarbons (TPH) in groundwater is based on the protection of adjacent surface waters, and is set at 10 milligrams per liter (mg/L) TPH (for diesel and oil). The cleanup level for TPH in soil was identified as 200 milligrams per kilogram (mg/kg). Alternatively, compliance with the soil cleanup level can be demonstrated empirically by documenting compliance with groundwater cleanup standards. The point of compliance for groundwater is within the nonaqueous phase liquid (NAPL) area, and at the downgradient property boundary. The cleanup actions selected to achieve these cleanup standards included capping the site, installing and operating a groundwater and NAPL recovery system, and monitoring.

1.2 Additional Studies Rationale

The Work Plan was developed to evaluate compliance with the cleanup standards in accordance with the five-year review requirement in the PPCD. Groundwater sampling performed in 2004 showed—with one exception—that TPH concentrations in groundwater were below cleanup

levels. The 2004 groundwater sample from well CW-5 (which also contained NAPL) contained TPH (13 mg/L) above the 10 mg/L cleanup level. Samples from the remediation system effluent have also been below the cleanup level for the last 21 sampling events, and below the laboratory detection limit during August 2007. These data suggest that groundwater cleanup standards under the PPCD may have been reached.

A NAPL recovery system was installed during 1997 and operated through 2007, resulting in a decrease in the number of wells with measurable accumulations of NAPL. Figure 2 shows the current Site, locations of monitoring wells, and also depicts when measurable accumulations of NAPL were last observed. While the extent of measurable NAPL appears to have decreased, measurable NAPL remains in wells CW-4 and CW-5. NAPL recovery data show that the system produces relatively little NAPL compared to the total volume of groundwater pumped (less than 0.3 percent by volume), and since 2004 the recovery rate and volume recovered have substantially declined. The NAPL is black in color and highly viscous, suggesting that the NAPL is depleted of water-soluble constituents, with little potential to migrate further or contaminate groundwater or surface water.

Based on the results of the groundwater monitoring and operation of the remediation system, Ecology agreed that additional data were necessary to assess the progress of remedial action and compliance with PPCD cleanup standards. Direct observation methods were also proposed to document NAPL in subsurface soil, so that NAPL accumulations in monitoring wells could be directly compared to NAPL present in the adjacent soil.

1.3 Nature of NAPL in the Subsurface

The behavior of NAPL in the subsurface is discussed below and is presented in greater detail in Appendix A (from the American Petroleum Institute [API] Interactive LNAPL [light nonaqueous phase liquid] Guide, 2004)

A common misconception among remediation practitioners is that NAPL in the subsurface exists as a homogeneous, separate-phase lens of NAPL at the water table. This "pancake" model led to several inaccurate conclusions about the occurrence, mobility, and recoverability of NAPL in the subsurface, including the following:

- The thickness of NAPL measured in a monitoring well is directly proportional to the amount of NAPL in the surrounding soil.
- Soils directly above the water table contain a uniformly high saturation of NAPL.
- If NAPL accumulations are observed in a well, NAPL in the adjacent formation is easily recoverable, and likely to migrate further.

• Soil and NAPL properties were not considered significant limiting factors in determining the mobility of NAPL.

A better model for NAPL in the subsurface considers the coexistence of NAPL, air, and water in the soil-pore throats. The degree of saturation of NAPL in the soil, and its mobility and recoverability, are highly dependent on the physical properties of the soil (e.g., heterogeneity, grain size, porosity, permeability) and NAPL (e.g., viscosity, composition, degree of weathering). In this multiphase model, NAPL partially fills the soil-pore space, and the degree of NAPL saturation decreases with depth until water fills all the pores (Beckett and Huntley, 1998). Though the NAPL saturation varies with depth, some of this NAPL may migrate into an adjacent monitoring well, with NAPL from the more permeable zones in the impacted soil column displacing water in a monitoring well, resulting in NAPL accumulations in the well that have no direct relationship to the amount of NAPL in the adjacent soil (Figure 3).

1.4 Scope of Work

The fieldwork activities and laboratory testing were performed in accordance with the Work Plan and consisted of three primary tasks:

- Task 1 consisted of groundwater sampling to measure the concentrations of petroleum hydrocarbons and related constituents for comparison to groundwater cleanup standards listed in the PPCD.
- Task 2 consisted of collecting undisturbed soil core samples within the NAPL zone for direct observation under regular and ultraviolet light. Selected soil samples were also tested for certain physical properties (e.g., grain-size distribution, porosity, permeability, air/water/NAPL content).
- Task 3 consisted of collecting NAPL samples for chemical and physical characterization. The purpose of the testing was to characterize the nature and degree of weathering of the petroleum hydrocarbons for comparison with previous results, and to measure the physical characteristics of the NAPL (e.g., density, viscosity, surface tension, and interfacial tension).

Ecology's letter approving the Work Plan contained the following additional requirements:

- 1. Perform additional groundwater sampling if the extraction system is not operated for more than three months.
- 2. Evaluate solubility and toxicity as part of the NAPL characterization.
- 3. Evaluate the potential for downward migration of petroleum compounds to the lower aquifer.

2.1 Site Description and History

The Site covers approximately 53 acres of industrially-zoned property within the Port of Tacoma (Figure 1). The Site is bordered by East 11th Street on the northwest, Milwaukee Way on the northeast, Lincoln Avenue on the southeast, and Cascade Pole and Lumber Company and the Maersk-Pacific marine terminal on the southwest. The surface topography is essentially even, with about a 5-foot crown near the center of the Site for surface drainage.

The Site was originally a tidal marsh within the Puyallup River delta. The Site was filled with dredged sediment from construction of nearby waterways in approximately 1900. By 1909, the Chicago, Milwaukee, Saint Paul, and Pacific Railroad Company had begun operating the Site as a railyard. The railyard was the West Coast headquarters for train maintenance and repairs. The railyard included maintenance and repair shops, and various other supporting buildings. The Site configuration changed very little between 1918 and 1980. Figure 4 presents a 1969 aerial photograph of the Site with key features noted, and the current monitoring well network added to the photograph. A former river channel west of the Site, shown on the 1969 aerial photograph, was filled in as part of industrialization of the area.

The railyard closed on March 15, 1980, and demolition began thereafter. The Union Pacific Railroad (UPRR) acquired the property in 1981. The Site has undergone redevelopment since the 1980s, including removal of all buildings/structures, underground and above-ground storage tanks and associated piping, and all other appurtenances associated with railyard operations. In 1995, Ecology and the Port of Tacoma entered into the PPCD, and the Port of Tacoma took ownership of the property later that year. A restrictive covenant prohibiting the use of groundwater for domestic purposes was also recorded by the Pierce County Auditor.

The surface water body closest to the Site at the time of the purchase of the UPRR property by the Port of Tacoma in 1995 was the Milwaukee Waterway. The Milwaukee Waterway was subsequently filled with dredged materials from the Sitcum and Blair Waterways, capped, and redeveloped as a marine terminal (the base map for Figure 1 was modified to show the current configuration of the marine terminal). At present, the nearest surface water bodies are the Sitcum Waterway and the Puyallup River.

Redevelopment of the Site was completed by the beginning of 2005. The Site has been capped with asphalt and utilized for vehicle and/or container storage (Figure 2). The Site is fenced in such a way that public access is prohibited and only controlled access is allowed.

2.2 Site Geology and Hydrogeology

The Site comprises hydraulically placed fill overlying fluvial and tidal sediments of the Puyallup River Delta. Prior to development, the entire tideflats area, including the Site, was a tidal marsh. The former tideflat surface can be observed in borings and test pits as a silt layer occurring beneath the overlying hydraulic fill. The geology beneath the Site consists of the following units (starting with the uppermost unit):

- The "Upper Sand" comprises 5 to 15 feet of fine- to coarse-grained silty sand (hydraulically placed fill), and typically occurs at grade to 10 feet below ground surface (bgs). The Upper Sand also contains areas of construction debris.
- A small portion of the former City of Tacoma landfill at the southeast end of the Site is present at the same depth as the Upper Sand, which covers the remainder of the Site.
- Beneath the Upper Sand, the "*Upper Silt*" represents the former tidal marsh sediments. This unit may range from a few inches to 4 or 5 feet thick, typically occurring at 10 to 12 feet bgs. The Upper Silt is absent along much of the northeast side of the Site.
- Beneath the Upper Silt, the "Lower Sand" comprises 15 to 25 feet of fine- to coarse-grained sand. This sand extends to approximately 30 to 35 feet bgs, and is in direct contact with the Upper Sand in areas where the Upper Silt is absent.
- Beneath the Lower Sand, the "Lower Silt" occurs at approximately 30 to 35 feet bgs, and is believed to be continuous beneath the Site and adjacent properties.

Groundwater occurs in the Upper Sand under water table (unconfined) conditions and in the Lower Sand under semiconfined conditions (lower aquifer). The Upper Silt is a significant aquitard where present, and the Lower Silt is an aquitard of Site-wide significance. Groundwater in the Upper and Lower Sands is in hydraulic communication where the Upper Silt is absent.

Groundwater recharge occurs from direct stormwater infiltration. Groundwater flow in the Upper Sand appears to be toward the area where the Upper Silt is absent, or towards the Lincoln Avenue ditch at the southeast end of the Site. The Lower Sand receives recharge from the Upper Sand.

2.3 Previous Investigations

In 1990, investigations performed on behalf of UPRR found NAPL, comprised predominantly of diesel fuel in the east-central portion of the Site along Milwaukee Way (Applied Geotechnology Inc. [AGI], 1990a; AGI, 1990b). In May 1990, a 6,000-gallon underground storage tank (UST), formerly used for storage of Bunker C fuel, was removed. Soils containing bunker oil were removed from the tank pit and piping trenches as an interim cleanup action.

A hydrologic characterization study was completed for the Site in 1993 (United States Pollution Control Inc. [USPCI], 1993). The results of the study identified concentrations of TPH and NAPL in soil and groundwater. Additional potential source areas identified were two former refueling racks; a product pipeline used to carry bunker fuel oil, and later diesel (abandoned in 1995); the round house area; and various USTs (USPCI, 1993). A study conducted in 1993 concluded that NAPL was not migrating to the Milwaukee Waterway (which was subsequently filled in 1995) or the Sitcum Waterway from the Site (AGI, 1993). A feasibility study was completed in 1994 that identified cleanup approaches for the Site (EMCON, 1994).

A remediation system consisting of 22 recovery wells was installed during 1997, and a total of approximately 63,700 gallons of NAPL has since been recovered (Shaw, 2008a). Since 2004, the recovery rate and volume recovered have declined, and the system produces mostly water. Most of the well-heads for the extraction wells are not accessible from the surface as a result of Site grading and paving that occurred after the system was installed. Water elevations and product thickness measurements are made from other monitoring wells, which were completed at ground surface during paving of the Site.

3.0 Results

This section presents the field and laboratory testing results. The field investigation and laboratory testing methods are described in Appendix B.

3.1 Task 1 – Groundwater Monitoring

Groundwater monitoring and sampling was performed in January and March 2008, and reported to Ecology in the January 2008 and April 2008 monthly progress reports (Shaw, 2008b and 2008c). Groundwater elevations and NAPL thicknesses measured in 2008, along with historical data, are presented in Table 1. Figure 2 also shows monitoring wells that have documented NAPL accumulations since 1993 (CW-1, CW-4, CW-5, CW-8, and MW-29). Measurable NAPL accumulations are now present in only two wells (CW-4 and CW-5) since September 2006.

Groundwater elevations and chemical data measured in January and March 2008, and groundwater elevation contours interpreted from these data, are shown in Figures 5 and 6, respectively. Both figures show groundwater generally flowing toward the north-northeast, with a horizontal hydraulic gradient of approximately 0.002 feet per foot. The gradient and flow direction are consistent with groundwater flow maps developed since the early 1990s.

A summary of the groundwater analytical results for samples collected in January and March 2008, along with results for samples collected in August 2004 (CW-4 and CW-5 were not sampled in 2004), are provided in Table 2. The 2004 and 2008 results indicate that diesel- and oil-range TPH were either not detected or detected at concentrations ranging from 0.13 to 13 mg/L. The detected concentrations are below the TPH cleanup level of 10 mg/L established for the Site under the PPCD, with the exception of a single sample result from well CW-5 (January 2008 sample at 13 mg/L as diesel). Additionally, the January and March 2008 data indicate benzene, toluene, ethyl benzene, and total xylenes (BTEX), polynuclear aromatic hydrocarbons (PAHs), naphthalene, and polychlorinated biphenyls (PCBs) were not detected in offsite compliance wells MW-21, MW-22, and MW-23, which are located downgradient or crossgradient from the area where NAPL was previously documented.

3.2 Task 2 - Test Boring and Soil Characterization

Two test borings (SC-1 and SC-2) were advanced to collect soil samples for physical observation and testing in areas with documented NAPL (Figures 2 and 4). Boring SC-1 was drilled near well CW-8, where NAPL was last observed in 2002. Boring SC-2 was drilled near well CW-5, where measurable accumulations of NAPL were present during this investigation. Soil cores were photographed under natural and ultraviolet light by PTS Laboratories in Santa Fe Springs, California, to identify NAPL. Selected soil samples were also tested for physical properties. The

methodology for collecting and testing the samples is described in Appendix B. Soil boring logs are presented in Appendix C. Laboratory reports and sample chain-of-custody records are included in Appendix D.

Annotated photographic logs divided into 2-foot intervals for SC-1 are provided in Figures 7 through 11, and for SC-2 in Figures 12 through 15. Photographs of the drilling and soil coring activities are provided in Appendix E.

3.2.1 Test Boring SC-1

Fill was encountered to depths of approximately 5 to 6 feet bgs. The fill material was then underlain by the Upper Sand (consisting generally of brown and gray-brown to gray medium dense, very fine to medium sand with some pebbles and rare gravels, trace fines, and occasional sandy silt or sandy silt zones The Upper Silt was not encountered to the total depth drilled (21 feet bgs).

Grain size testing (Table 3), hydraulic conductivity measurements (Table 4) and capillary pressure curves (Figure 16) are indicative of fine to medium sand material.

The laboratory photographs of the cores (Figures 7 through 11) exhibit fluorescence consistent with the presence of NAPL in finely laminated zones at depths of approximately 8.0 to 8.3, 8.6 to 9.0, and 11 to 12.3 feet bgs (in the photographs, NAPL fluoresces yellowish-green; the intensity of the fluorescence increases with the amount of NAPL present).

Table 5 shows that pore fluid is primarily saturated with water, with comparatively small amounts of NAPL (2.9 percent to 8.8 percent). Table 6 shows that NAPL was not extractable from the soil-pore throats during centrifuging at 1,000 times the acceleration due to gravity.

3.2.2 Test Boring SC-2

The soil profile encountered in Test Boring SC-2 was very similar to SC-1, consisting of fill underlain by the Upper Sand. The Upper Sand consisted generally of fine to medium sand with zones of gravel and coarse-grained sand. The gravel zones decrease with depth and are most abundant to a depth of approximately 12.0 feet bgs. The Upper Silt was not encountered to the total depth drilled (20 feet bgs).

Grain size testing (Table 3), hydraulic conductivity measurements (Table 4) and capillary pressure curves (Figure 16) are consistent with fine to coarse sand.

The laboratory photographs of the cores (Figures 12 through 15) show NAPL from approximately 7.9 feet to the total depth sampled (17 feet bgs). The greatest amount of NAPL (i.e., highest fluorescence) occurs from approximately 15.3 to 17 feet bgs, and is present in the sandy layers and not present in the more permeable gravel interbeds.

Table 5 shows that pore fluid consists primarily of water, except for the samples obtained from a depth interval of 16 to 17 feet bgs where NAPL and water saturations were roughly equivalent. Table 6 suggests that the NAPL is extractable from the soil-pore throats during centrifuging at 1,000 times the acceleration due to gravity.

3.3 Task 3 - NAPL Chemical and Physical Characterization

NAPL and groundwater samples were collected on June 7 and 8, 2007, from five wells (RW2L3, RW5L3, RW2L5, CW-4, and CW-5) and submitted to Torkelson Geochemistry, Inc. (TGI) of Tulsa, Oklahoma, for chemical and physical testing. Geochemical and physical properties test results are summarized on Table 7. TGI's evaluation report, including all gas chromatograms, is provided in Appendix F.

The TGI results show that NAPL at the Site has undergone biological, chemical, and/or physical alteration resulting from exposure to the natural environment (i.e., weathering). Compositions range from weathered diesel (middle distillate) to weathered heavy lube oil-range petroleum hydrocarbons. A chromatogram representative of the NAPL collected from well RW-2L3 and a chromatogram for an unweathered diesel standard are presented in Figure 17. A summary of the effects of NAPL weathering and chromatogram interpretation is presented below.

Gas chromatography (GC) is a primary method for identifying NAPL constituents. NAPL is injected into the GC, and enters a small-diameter coiled capillary tube. As the sample is heated and travels through the capillary tube, NAPL constituents separate by polarity and boiling point. A chromatogram depicts NAPL constituents as they exit the capillary tubing. The chromatograms identify the relative abundance (as peaks depicted on the y-axis) of the individual compounds, and constituents are identified by retention time (depicted as time beneath a peak on the x-axis). Lighter (lower molecular weight) compounds with shorter retention times appear on the left-hand side of a chromatogram. Heavier (higher molecular weight) compounds have longer retention times and appear on the right-hand side of a chromatogram. There are several "marker" compounds that indicate relative degrees of weathering, particularly in diesel-range hydrocarbons. Selected marker compounds are labeled on the chromatogram in Figure 17.

A chromatogram of non-weathered diesel fuel is characterized by an array of repeating peaks of normal alkanes (i.e., a homologous series of compounds including octane [nC8], nonane [nC9], and decane [nC10]). These compounds characteristically appear as the dominant peaks, and are evenly distributed in the middle range of the diesel standard (Figure 17). These compounds are easily weathered and decrease in relative abundance compared to other compounds with increasing weathering. This decrease in normal alkanes accentuates the relative abundance of other marker compounds that are more persistent (i.e., the isoprenoid compounds, labeled as IP12, IP14, etc.). Two specific marker isoprenoids are pristane and phytane, which are located next to nC17 and nC18 alkane peaks. With increasing weathering, these isoprenoid compounds

increase relative to the adjacent normal alkanes (Bruce and Schmidt, 1994). The TGI chromatograms show that all of the NAPL samples are so degraded that the normal alkanes are nearly absent on all the GC chromatograms, and the only dominant identifiable peaks are isoprenoids, particularly pristane and phytane (Figure 17).

TGI's analyses show that the NAPL consists of variable amounts of degraded diesel with some heavier lube-oil range hydrocarbons (see Appendix F). The NAPL sampled from RW-2L3 is primarily degraded diesel, while RW-5L3 contains degraded diesel and heavier lube-oil range hydrocarbons. The remainder of NAPL samples indicated a composition intermediate between RW-2L3 and RW-5L3, and so samples RW-2L3 and RW-5L3 were also tested for density and viscosity (Table 7). The density of the NAPL from RW-2L3 of 0.8803 gram/milliliter (gm/ml) is consistent with weathered diesel (non-degraded diesel has a density of about 0.827 gm/ml; API, 2004). The NAPL sample collected from well RW-5L3 has a density of 0.9498 gm/ml, which is consistent with a heavy lube or fuel oil. The viscosity of the RW2L3 sample (8.34 centipoise [cP]) is comparable to the viscosity of No. 2 fuel oil (4.04 cP), and the viscosity of RW-5L3 (633 cP) is comparable No. 6 fuel oil (Bunker C) and other heavier fuels (122 cP to 3,180 cP) (Beckett and Huntley, 2002).

4.0 Summary and Conclusions

Groundwater sampling and NAPL testing indicate the presence of highly weathered petroleum that has been depleted of water-soluble hazardous constituents. Soil testing results also show that the residual NAPL is typically absent from the pore throats of coarser-grained soil, but is present in finer-grained soils which are less amenable to NAPL removal.

Most of the fluid that is pumped by the remediation system consists of water extracted from coarser-grained soils. Recovery data show that the remediation system recovers less than 0.3 percent NAPL by volume.

The results of groundwater monitoring data collected at the Site show that the groundwater cleanup standards have been met and that the remaining residual NAPL is not impacting groundwater. All of the groundwater samples were either non-detect or below the cleanup levels for petroleum compounds, with the exception of one sample from well CW-5, which was marginally above the cleanup level for TPH in January 2008. BTEX, carcinogenic PAHs, naphthalene, and PCBs were not detected in groundwater samples from offsite compliance wells MW-21, MW-22, and MW-23, which are located downgradient or crossgradient from the area that has historically been affected by NAPL. Since dissolved constituents are generally not partitioning from the NAPL to the groundwater in the area downgradient of or below the NAPL, migration to the deeper aquifer is unlikely.

Shaw recommends sampling wells CW-4 and CW-5 for three consecutive quarters to ascertain compliance with the cleanup standards associated with the PPCD. Recommendations for further work should be based on the results of these additional data.

5.0 References

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Limitations

The services described in this report were performed consistent with generally accepted professional consulting principles and practices. No other warranty, expressed or implied, is made. These services were performed consistent with our agreement with our client. This report is solely for the use and information of our client unless otherwise noted. Any reliance on this report by a third party is at such party's sole risk.

Opinions and recommendations contained in this report apply to conditions existing when services were performed and are intended only for the client, purposes, locations, time frames, and project parameters indicated. We are not responsible for the impacts of any changes in environmental standards, practices, or regulations subsequent to performance of services. We do not warrant the accuracy of information supplied by others, or the use of separated portions of this report.

Tables

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-1	11/28/96	17.81	8.06	NP	9.75
	01/27/97		5.46	0.04	12.39*
	05/22/97		7.30	0.05	10.55*
	10/10/97		8.04	0.08	9.84*
	10/31/97		8.42	0.10	9.48*
	11/07/97		7.86	0.25	10.17*
	01/14/98		6.38	0.07	11.49*
	02/26/98		6.26	0.43	11.93*
	04/02/98		6.68	Film	11.13
	04/30/98		7.52	0.01	10.30*
	06/02/98		7.81	1.58	11.41*
	06/30/98		8.36	0.22	9.65*
	07/23/98		8.65	0.30	9.43*
	08/31/98		9.11	0.45	9.10*
	09/30/98		9.31	0.45	8.90*
	10/29/98		9.40	0.42	8.78*
	11/30/98		6.24	NP	11.57
	12/30/98		5.48	NP	12.33
	01/29/99		5.21	NP	12.60
	02/25/99		5.41	NP	12.40
	03/31/99		6.42	NP	11.39
	04/30/99		7.34	Film	10.47
	06/02/99		7.89	NP	9.92
	06/30/99		8.04	NP	9.77
	07/14/99		8.13	NP	9.68
	08/18/99		8.39	NP	9.42
	09/15/99		8.66	Film	9.15
	10/28/99		9.18	0.08	8.70*
	11/24/99		7.00	NP	10.81
	12/21/99		6.64	NP	11.17
	01/18/00		6.94	0.22	11.17
	03/01/00		6.56	NP	11.07
İ	03/01/00		6.88	NP	
	1				10.93
	04/26/00		7.58	Film	10.23
	05/31/00		8.02	NP ND	9.79
	06/29/00		8.16	NP ND	9.65
	07/25/00 08/15/00		8.38	NP ND	9.43
	i		8.59	NP	9.22
	09/12/00		8.84	NP	8.97
	10/17/00		9.24	0.22	8.77*
	11/14/00		8.48	0.03	9.36*
	12/29/00		8.11	0.02	9.72*
ļ	01/19/01		8.01	0.03	9.83*
	02/27/01		7.91	0.01	9.91*
	03/26/01		8.28	0.01	9.54*
	04/30/01		8.06	0.01	9.76*
	05/28/01		7.94	0.01	9.88*
	06/25/01		8.29	0.01	9.53*
	07/23/01	Ī	8.46	0.01	9.36*
1	08/27/01		9.42	0.01	8.40*
	09/23/01		8.65	0.01	9.17*

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-1, cont.	10/25/01		8.75	0.01	9.07*
	11/29/01		6.30	0.01	11.52*
	12/29/01		6.25	NP	11.56
	01/31/02		6.14	NP	11.67
	02/28/02		6.57	Film	11.24
	03/28/02		6.79	NP	11.02
	04/24/02		7.03	NP	10.78
	05/30/02		7.80	NP	10.01
	06/27/02		8.30	NP	9.51
	07/30/02		8.48	NP	9.33
	08/29/02		8.72	Film	9.09
	09/27/02		8.92	Film	8.89
	10/31/02		9.16	0.01	8.66*
	11/26/02		8.70	0.01	9.12*
	12/26/02		7.52	0.03	10.32*
	01/30/03		6.02	NP	11.79
	02/27/03		6.86	NP	10.95
	03/31/03		6.76	NP	11.05
	04/28/03		7.14	NP	10.67
	05/29/03		7.74	NP	10.07
	06/25/03		8.08	NP	9.73
	07/30/03		8.47	Film	9.34
	08/28/03		8.66	NP	9.15
	09/29/03		8.93	Film	8.88
	10/29/03		7.40	NP	10.41
	11/25/03		7.33	NP	10.48
	01/09/04		6.53	NP	11.28
	01/27/04		6.61	NP	11.20
	02/26/04		6.78	NP	11.03
	03/29/04		7.41	Film	10.40
	04/30/04		7.91	NP	9.90
	05/25/04		8.11	NP	9.70
	06/30/04		8.41	NP	9.40
	07/28/04		8.64	NP	9.17
	08/25/04		8.77	NP	9.04
	09/29/04		8.56	NP	9.25
	10/28/04		8.28	NP	9.53
	11/23/04		8.21	NP	9.60
	12/30/04	ļ	7.55	NP	10.26
ļ	01/31/05		7.21	NP	10.20
	02/28/05		7.99	NP	9.82
	03/30/05		7.48	NP	10.33
j	03/30/03		7.51	NP	10.33
	04/2//03		6.88	NP	10.93
	05/26/05		7.22	NP	10.53
ļ			8.02	NP NP	9.79
	07/26/05				9.79
	09/08/05		8.50	0.01 ND	9.31
	09/30/05		8.71	NP ND	
	10/28/05		8.48	NP ND	9.33
	11/30/05 12/28/05		7.85 6.85	NP NP	9.96 10.96

F	T	T		In	
337 11 57 1	D . G !!	Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-1, cont.	01/27/06		6.06	Film	11.75
	02/27/06		6.74	NP	11.07
	03/28/06		7.06	NP	10.75
	04/25/06		7.14	NP	10.67
	05/30/06		7.54	NP	10.27
	06/27/06		7.64	NP	10.17
	07/24/06		8.02	NP	9.79
	08/30/06		8.39	NP	9.42
	09/29/06		8.77	NP	9.04
	10/31/06		8.79	NP	9.02
	11/30/06		6.60	NP	11.21
	12/28/06		5.86	NP	11.95
	01/31/07		6.68	NP	11.13
	02/28/07		6.63	NP	11.18
	03/28/07		6.42	NP	11.39
	04/30/07		7.23	NP	10.58
	05/23/07		7.61	NP	10.20
	06/29/07		7.98	NP	9.83
	07/30/07		8.01	NP	9.80
	08/29/07		8.32	NP	9.49
	09/27/07		8.57	NP	9.24
	10/25/07		7.92	NP	9.89
	11/28/07		8.05	NP	9.76
	01/09/08		6.87	NP	10.94
	02/13/08		6.85	NP	10.96
	03/19/08		7.45	NP	10.36
CW-2	02/28/07	20.44	8.61	NP	11.83
	03/28/07		8.27	NP	12.17
	04/30/07		8.87	NP	11.57
	05/23/07		9.28	NP	11.16
	06/29/07		9.82	NP	10.62
	07/30/07		9.95	NP	10.49
	08/29/07		10.23	NP	10.21
	09/27/07		10.53	NP	9.91
	10/25/07		9.97	NP	10.47
	11/28/07		10.00	NP	10.44
1	01/09/08	l	8.95	NP	11.49
	02/13/08		8.58	NP	11.86
	03/19/08		9.26	NP	11.18
CW-3	02/28/07	19.31	6.44	NP	12.87
	03/28/07	17.51	6.27	NP	13.04
	04/30/07		7.24	NP	12.07
	05/23/07		7.74	NP	11.57
	06/29/07		8.40	NP	10.91
	07/30/07	ļ	8.55	NP	10.76
	1			l l	
	08/29/07		8.85	NP ND	10.46
	09/27/07		9.19	NP ND	10.12
	10/25/07		8.67	NP ND	10.64
	11/28/07		8.60	NP	10.71
	01/09/08		7.60	NP	11.71

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-3, cont.	02/13/08		7.30	NP	12.01
	03/19/08		7.91	NP	11.40
CW-4	11/28/96	16.50	8.14	2.28	10.39*
	01/27/97		3.65	0.26	13.08*
	05/22/97		5.71	0.26	11.02*
	10/10/97		6.67	1.06	10.77*
	10/31/97		7.03	0.97	10.33*
	11/07/97		6.00	0.19	10.67*
	01/14/98		4.35	Film	12.15
	02/26/98		5.12	Film	11.38
	04/02/98		5.04	0.23	11.66*
	04/30/98		6.77	0.96	10.58*
	06/02/98		7.67	2.25	10.83*
	06/30/98		—	>0.50	
	07/23/98		—	>0.50	
	08/31/98		14.26	7.19	8.64*
	09/30/98			>0.50	_
	10/29/98		14.10	6.68	8.35*
	11/30/98		8.11	2.05	10.21*
-	12/30/98		5.01	1.32	12.66*
	01/29/99		3.11	1.95	15.13*
	02/25/99		4.80	1.65	13.17*
	03/31/99		6.80	2.35	11.79*
	04/30/99		6.78	1.14	10.73*
	06/02/99		9.35	3.27	10.06*
	06/30/99			>5.00	
	07/14/99		11.70	5.27	9.49*
	08/18/99		14.00	6.70	8.46*
	09/15/99		14.00	6.92	8.66*
	10/28/99		14.70	6.70	7.76*
	11/24/99		5.42	Film	11.08
	12/21/99		5.17	Film	11.33
	01/18/00		5.34	0.25	11.38*
	03/01/00		5.33	0.80	11.88*
	03/30/00		5.50	0.37	11.33*
	04/26/00		8.20	2.40	10.44*
	05/31/00		9.82	3.51	9.80*
	06/29/00		14.10	7.64	9.20*
	07/25/00		14.00	7.05	8.77*
	08/15/00		13.51	6.71	8.96*
ŀ	09/12/00		13.60	6.61	8.78*
	10/17/00		13.70	6.06	8.19*
	11/14/00		13.78	6.79	8.76*
	12/29/00		7.65	1.39	10.09*
	01/19/01		9.38	2.89	9.69*
	02/27/01		14.95	8.21	8.86*
	03/26/01		NM	NM	NM
	04/30/01		8.48	1.88	9.69*
	05/28/01		15.45	8.05	8.21*
	06/25/01		7.99	0.91	9.32*

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
,, on reamour	Date Sollected	(feet)	(feet)	(feet)	(feet)
CW-4, cont.	07/23/01	(1001)	7.47	0.28	9.28*
C W -4, Cont.	08/27/01		9.96	0.65	7.12*
	09/23/01		8.56	1.41	9.19*
	10/25/01		7.90	0.11	8.70*
	11/29/01		5.26	0.51	11.69*
	12/29/01		6.85	0.12	9.76*
	01/31/02		7.00	0.12	10.30*
	02/28/02		5.05	0.30	11.66*
	03/28/02		5.90	0.24	10.69*
	04/24/02		7.70	1.80	10.40*
	05/30/02		8.90	0.98	8.47*
	06/27/02		11.00	3.40	8.53*
	07/30/02		7.60	0.08	8.97*
			9.00	0.08	8.12*
	08/29/02 09/27/02		9.00 8.26	Film	8.12**
	10/31/02		8.26 8.75	NP	7.75
			8.73	NP	8.19
	11/26/02		11.14	4.34	9.22*
	12/26/02 01/30/03		6.20	4.54 NP	10.30
	l l		7.00	1.00	10.30
	02/27/03 03/31/03		7.00 NM	NM	10.39 · NM
	i i		8.35	2.20	10.11*
	04/28/03		8.33 8.70	1.80	9.40*
	05/29/03		8.70 NM	NM	9.40 NM
	06/25/03		7.50	0.00	9.00
	07/30/03 08/28/03		8.30	0.50	9.00 8.65*
			7.90	0.30	8.69*
	09/29/03		7.90 7.01	NP	9.49
	10/29/03		6.88	NP NP	9.49
	11/25/03		5.20	0.10	11.39*
	01/09/04		5.35	NP	11.39
	01/27/04 02/26/04		3.33 7.95	2.81	11.13
	1		6.30	Film	10.20
	03/29/04 04/30/04		0.30 7.70	Film Film	8.80
	05/25/04		7.70 7.75	Film	8.75
	i i		9.50	1.50	8.73 8.34*
	06/30/04		9.30 8.20	0.79	9.00*
	07/28/04		8.20 NM	NM	9.00 · NM
	08/25/04		NM	NM NM	NM NM
	09/29/04				
	10/28/04		NM 8.90	NM 1.80	NM 9.20*
ļ	11/23/04		8.90 NM ^{b,c}	1.80 NM ^{b,c}	
	12/30/04		NM ^{b,c}	NM h,c	NM NM
	01/31/05		NM ^{b,c}	NM NM ^{b,c}	
	02/28/05				NM NA ^b
	03/30/05		10.53	0.05	
	04/27/05		10.65	0.31	NA ^b
ļ	05/26/05	20.50	10.55	0.41	NA ^b
	06/28/05	20.53	10.71	0.35	10.13*
	07/26/05		10.83	1.12	10.70*
	09/08/05		NM ^c	NM ^c	NM ^c
	09/30/05		13.28	2.25	9.25*

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
Well Indiliner	Date Confected	(feet)	(feet)	(feet)	(feet)
CW-4, cont.	10/28/05		13.48	2.17	8.98*
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	11/30/05		13.85	3.13	9.47*
	12/28/05		NM	NM	NM
	01/27/06		NM	NM	NM
	02/27/06		NM	NM	NM
	03/28/06		9.96	1.81	12.18*
	04/25/06		10.18	0.10	10.44*
	05/30/06		10.37	0.09	10.24*
	06/27/06		11.30	0.86	10.00*
	07/24/06		13.23	2.38	9.42*
	08/30/06		12.10	1.01	9.33*
	09/29/06		14.30	2.93	8.84*
	10/31/06		12.54	0.88	8.77*
	11/30/06		9.59	0.01	10.95*
	12/28/06		9.35	0.13	11.30*
	01/31/07		10.02	0.38	10.85*
	02/28/07		9.74	0.06	10.84*
	03/28/07		9.03	0.06	11.55*
	04/30/07		10.29	0.28	10.49*
	05/23/07		10.39	0.16	10.28*
	06/29/07		13.38	2.58	9.45*
	07/30/07		10.25	1.46	11.58*
	08/29/07		11.58	0.67	9.55*
	09/27/07		NM ^c	NM ^c	NM ^c
	10/25/07		13.11	2.25	9.42*
	11/28/07		11.00	1.00	10.42*
	01/09/08		16.15	5.26	9.06*
	02/13/08		10.00	0.35	10.84*
	03/19/08		13.20	3.06	10.05*
CW-5	11/28/96	18.76	8.85	0.71	10.54*
	01/27/97		10.65	5.81	13.28*
	05/22/97		9.00	1.70	11.27*
	10/10/97		7.66	0.01	11.11*
	10/31/97		7.76	0.01	11.01*
	11/07/97		7.78	0.01	10.99*
	01/14/98		6.82	0.05	11.98*
	02/26/98			>0.84	
	04/02/98		7.13	0.51	12.08*
	04/30/98	İ	7.69	0.01	11.08*
	06/02/98		8.42	0.86	11.11*
	06/30/98			>0.50	
İ	07/23/98		_	>0.50	
	08/31/98		12.73	3.37	9.03*
	09/30/98			>0.50	0.02*
	10/29/98		13.29	3.78	8.83*
	11/30/98		5.35	1.05	14.34*
	12/30/98		5.74	0.70	13.64*
	01/29/99		5.01	0.65	14.33*
	02/25/99		5.70	1.49	14.39*
	03/31/99		7.10	1.54	13.03*

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-5, cont.	04/30/99		8.20	1.34	11.75*
	06/02/99		8.29	0.78	11.16*
	06/30/99		9.65	0.95	9.96*
	07/14/99		8.32	1.88	12.11*
	08/18/99		9.95	1.13	9.82*
	09/15/99		9.92	0.71	9.47*
	10/28/99		9.89	0.17	9.02*
	11/24/99		9.90	3.09	11.61*
	12/21/99		7.69	0.79	11.77*
	01/18/00		7.80	1.19	12.02*
	03/01/00		7.42	1.24	12.44*
	03/30/00		10.80	3.70	11.25*
	04/26/00		8.30	0.25	10.68*
	05/31/00		10.45	2.01	10.10*
	06/29/00		12.65	4.20	9.85*
	07/25/00		12.70	3.12	8.84*
	08/15/00		11.47	2.02	9.09*
	09/12/00		11.61	1.73	8.69*
	10/17/00		11.65	1.68	8.61*
	11/14/00		9.06	Film	9.70*
	12/29/00		7.84	0.08	10.99*
	01/19/01		9.32	0.91	10.25*
	02/27/01		9.51	1.79	10.84*
	03/26/01		8.99	0.04	9.81*
	04/30/01		7.73	0.12	11.14*
	05/28/01		8.85	1.26	11.03*
	06/25/01		9.24	0.21	9.71*
	07/23/01		9.40	0.07	9.42*
	08/27/01		10.13	1.34	9.82*
	09/23/01		11.90	2.84	9.39*
	10/25/01		11.09	2.86	10.22*
	11/29/01		7.90	1.69	12.36*
	12/29/01		7.59	0.88	11.95*
	01/31/02		7.05	1.44	12.99*
	02/28/02		7.03 8.90	2.57	12.15*
	03/28/02		8.10	1.70	12.17*
	04/24/02		7.50	1.75	12.17*
	i		9.80	2.27	10.98*
	05/30/02		9.80 10.91	2.59	10.98*
	06/27/02				
	07/30/02		10.60	1.85	9.81*
	08/29/02		11.65	2.40	9.25*
	09/27/02		11.95	2.50	9.04*
	10/31/02		12.45	2.67	8.69*
	11/26/02		11.35	2.28	9.44*
	12/26/02		10.23	2.95	11.16*
	01/30/03		7.80	2.04	12.78*
	02/27/03		11.60	4.74	11.38*
	03/31/03		9.80	3.00	11.63*
	04/28/03		10.30	3.18	11.29*
	05/29/03		NM	NM	NM
	06/25/03		10.45	1.00	9.20*

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-5, cont.	07/30/03		11.80	1.90	8.65*
	08/28/03		11.80	4.53	10.99*
	09/29/03		14.10	4.35	8.53*
	10/29/03		8.57	1.13	11.20*
	11/25/03		8.34	0.95	11.27*
	01/09/04		7.90	1.30	12.02*
	01/27/04		9.76	3.35	11.98*
	02/26/04		7.70	1.05	11.99*
	03/29/04		8.85	1.57	11.31*
	04/30/04		10.30	2.31	10.52*
	05/25/04		10.23	1.63	9.98*
	06/30/04		11.40	2.58	9.66*
	07/28/04		10.70	2.10	9.93*
	08/25/04		9.90	0.85	9.62*
	09/29/04		10.91	1.46	9.15*
	10/28/04		10.20	1.65	10.03*
	11/23/04		10.20	1.60	9.98*
	12/30/04		NM ^{b,c}	NM ^{b,c}	NM
	01/31/05		14.30	1.50	NA^b
	02/28/05		NM ^{b,c}	NM ^{b,c}	NM
	03/30/05		12.30	2.92	NA ^b
	04/27/05		9.55	1.16	NA ^b
	05/26/05		9.14	1.10	NA ^b
	06/28/05	19.32	9.61	0.94	10.55*
	07/26/05	17,52	11.11	0.88	8.99*
	09/08/05		11.42	1.54	9.27*
	09/30/05		11.53	1.55	9.17*
	10/28/05		11.43	1.38	9.12*
	11/30/05		10.15	0.69	9.78*
	12/28/05		8.04	0.09	11.45*
			7.28	0.19	12.55*
	01/27/06		7.28	0.67	12.33*
	02/27/06				11.63*
	03/28/06		8.89	1.35	
	04/25/06	İ	8.88	1.59	11.86*
	05/30/06		8.45	0.41	11.23*
	06/27/06		8.80	0.44	10.91*
	07/24/06		9.84	0.68	10.09*
	08/30/06		11.80	2.23	9.50*
	09/29/06		10.80	0.85	9.28*
	10/31/06		12.27	0.38	7.39*
	11/30/06		8.13	0.74	11.85*
	12/28/06		7.44	0.72	12.52*
	01/31/07		NA	NA	NA
	02/28/07		NA	NA	NA
	03/28/07		7.81	0.97	12.37*
	04/30/07		NA	NA	NA
	05/23/07		NA	NA	NA
	06/29/07		10.21	0.62	9.66*
	07/30/07		11.51	2.12	9.70*
	08/29/07	j	10.22	0.64	9.67*
	09/27/07		11.01	0.22	8.51*

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-5, cont.	10/25/07		10.22	0.91	9.91*
,	11/28/07		9.25	0.20	10.25*
	01/09/08		10.50	2.45	11.00*
	02/13/08		11.87	4.22	11.21*
	03/19/08		11.47	3,31	10.80*
CW-6	02/28/07	20.52	10.22	NP	10.30
	03/28/07		10.24	NP	10.28
	04/30/07		10.89	NP	9.63
	05/23/07		11.04	NP	9.48
	06/29/07		11.21	NP	9.31
	07/30/07		11.22	NP	9.30
	08/29/07		11.52	NP	9.00
	09/27/07		11.74	NP	8.78
	10/25/07		11.38	NP	9.14
	11/28/07		11.25	NP	9.27
	01/09/08		10.32	NP	10.20
	02/13/08		10.59	NP	9.93
	03/19/08		10.99	NP	9.53
CW-7	02/28/07	19.81	9.02	NP	10.79
	03/28/07		9.04	NP	10.77
	04/30/07		9.54	NP	10.27
	05/23/07		9.72	NP	10.09
	06/29/07		9.83	NP	9.98
	07/30/07		9.90	NP	9.91
	08/29/07		10.16	NP	9.65
	09/27/07		10.41	NP	9.40
	10/25/07		10.11	NP	9.70
	11/28/07		10.00	NP	9.81
	01/09/08		9.15	NP	10.66
	02/13/08		9.38	NP	10.43
	03/19/08	10.50	9.74	NP	10.07
CW-8	11/28/96	18.28	8.63	0.12	9.76*
	01/27/97		7.54	2.03	12.55*
	05/22/97		8.60	0.80	10.39*
	10/10/97		8.61	0.14	9.79*
	10/31/97		8.78	0.11	9.60*
	11/07/97		8.30	0.01	9.99*
	01/14/98 02/26/98		7.00 7.71	Film 0.03	11.28 10.60*
ļ	02/26/98		7.71	0.03	10.00**
	04/02/98		8.96	0.13	10.97*
	06/02/98		8.68	0.83	9.88*
	06/30/98		9.17	0.32	9.43*
	07/23/98		9.38	0.41	9.26*
	08/31/98		9.96	0.67	8.92*
	09/30/98		10.13	0.72	8.79*
	10/29/98		10.40	0.91	8.69*
	11/30/98		6.73	0.20	11.73*
	12/30/98		6.48	0.39	12.15*
	01/29/99		6.27	0.58	12.53*

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-8, cont.	02/25/99		6.50	0.75	12.45*
	03/31/99		7.39	0.68	11.50*
	04/30/99		8.48	0.46	10.21*
	06/02/99		8.68	0.15	9.73*
	06/30/99		8.81	0.13	9.59*
	07/14/99		8.91	0.04	9.41*
	08/18/99		9.20	0.06	9.13*
	09/15/99		9.41	0.03	8.90*
	10/28/99		10.75	0.15	7.66*
	11/24/99		7.83	0.10	10.54*
	12/21/99		7.78	0.57	11.01*
	01/18/00		7.92	0.82	11.09*
	03/01/00		7.79	0.38	10.83*
	03/30/00		8.53	0.74	10.41*
	04/26/00		8.96	0.59	9.85*
:	05/31/00		9.20	0.41	9.44*
	06/29/00		8.95	0.10	9.42*
	07/25/00		9.39	0.21	9.08*
	08/15/00		9.61	0.32	8.95*
	09/12/00		9.81	0.40	8.83*
	10/17/00		10.64	1.20	8.71*
	11/14/00		9.88	0.57	8.91*
	12/29/00		8.96	0.09	9.40*
	01/19/01		8.85	0.09	9.51*
	02/27/01		8.82	0.09	9.54*
	03/26/01		9.41	0.15	9.00*
	04/30/01		8.80	0.09	9.56*
	05/28/01		8.70	0.09	9.66*
	06/25/01	·	8.70	0.09	9.66*
	07/23/01		9.18	0.03	9.13*
	08/27/01		9.20	0.05	9.12*
İ	09/23/01		9.37	0.08	8.98*
	10/25/01		9.67	0.22	8.81*
]	11/29/01		7.21	0.02	11.09*
	12/29/01		6.95	0.02	11.36*
	01/31/02		6.72	NP	11.56
	02/28/02		7.10	0.01	11.19*
	03/28/02		7.35	0.01	10.94*
i	04/24/02		7.67	0.01	10.63*
	05/30/02		8.38	NP	9.90
	06/27/02		8.84	NP	9.44
	07/30/02		9.13	0.02	9.44
	08/29/02		9.13	0.02	9.17* 8.90*
	08/29/02	ļ	9.42	i	
	i		1	0.10	8.69* 8.52*
	10/31/02	l	9.95	0.22	8.53*
	11/26/02		9.53	0.10	8.84*
	12/26/02		8.12	NP	10.16
	01/30/03		6.32	NP	11.96
	02/27/03	ļ	7.54	NP	10.74
	03/31/03	i	7.42	NP	10.86

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
CW-8, cont.	05/29/03		8.63	NP	9.65
	06/25/03		8.97	NP	9.31
	07/30/03		9.20	Film	9.08
	08/28/03		9.31	NP	8.97
	09/29/03		9.55	Film	8.73
	10/29/03		8.12	NP	10.16
	11/25/03		8.17	NP	10.11
	01/09/04		7.51	NP	10.77
	01/27/04		7.29	NP	10.99
	02/26/04		7.51	Film	10.77
	03/29/04		8.20	NP	10.08
	04/30/04		8.72	NP	9.56
	05/25/04		8.82	NP	9.46
	06/30/04		9.00	NP	9.28
	07/28/04		9.12	NP	9.16
	08/25/04		9.24	NP	9.04
	09/29/04		9.18	NP	9.10
	10/28/04		8.80	NP	9.48
	11/23/04		8.95	NP	9.33
	12/30/04		13.05	NP	NA ^b
	01/31/05		12.72	Film	NA^b
	02/28/05		10.49	NP	NA ^b
	03/30/05		8.98	NP	NA ^b
	04/27/05		9.88	NP	NA ^b
	05/26/05		9.17	NP	NA ^b
	06/28/05	19.61	9.55	NP	10.06
	07/26/05	17.01	10.22	NP	9.39
	09/08/05		10.66	NP	8.95
	09/30/05		9.73	Film	9.88
	10/28/05		10.67	Film	8.94
	11/30/05		9.99	Film	9.62
	12/28/05		9.99 8.72	NP	10.89
	01/27/06		8.16	Film	11.45
			8.91	NP	10.70
	02/27/06		8.94	NP	10.70
	03/28/06				10.67
	04/25/06		9.33	NP ND	
	05/30/06		9.59	NP ND	10.02
	06/27/06		9.97	NP	9.64
	07/24/06		10.19	NP	9.42
	08/30/06		10.56	NP	9.05
	09/29/06		10.86	NP	8.75
	10/31/06		11.02	NP	8.59
	11/30/06		8.95	NP	10.66
	12/28/06		8.34	NP	11.27
	01/31/07		9.07	NP	10.54
	02/28/07		8.79	NP	10.82
	03/28/07		8.23	NP	11.38
	04/30/07		9.51	NP	10.10
ļ	05/23/07		9.68	NP	9.93
	06/29/07		10.19	NP	9.42
	07/30/07		10.21	NP	9.40

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected		Groundwater	Thickness	Elevation
well number	Date Collected	Elevation		Į.	I .
CW 0	00/00/07	(feet)	(feet)	(feet)	(feet)
CW-8, cont.	08/29/07	•	10.46	NP	9.15
	09/27/07		10.71	NP	8.90
	10/25/07		10.21	NP	9.40
	11/28/07		10.21	NP	9.40
	01/09/08		8.51	NP	11.10
	02/13/08		8.89	NP	10.72
	03/19/08		9.58	NP	10.03
CW-9	02/28/07	18.06	9.16	NP	8.90
	03/28/07		9.07	NP	8.99
	04/30/07		9.43	NP	8.63
	05/23/07		9.53	NP	8.53
	06/29/07		9.64	NP	8.42
	07/30/07		9.81	NP	8.25
	08/29/07		9.97	NP	8.09
	09/27/07		10.10	NP	7.96
	10/25/07		9.88	NP	8.18
	11/28/07		9.78	NP	8.28
	01/09/08		9.22	NP	8.84
	02/13/08		9.18	NP	8.88
	03/19/08		9.59	NP	8.47
MW-7	02/28/07	19.85	5.87	NP	13.98
	03/28/07		5.67	NP	14.18
	04/30/07		6.76	NP	13.09
	05/23/07		7.25	NP	12.60
	06/29/07		8.10	NP	11.75
	07/30/07		8.21	NP	11.64
	08/29/07		8.49	NP	11.36
	09/27/07		8.78	NP	11.07
	10/25/07		8.04	NP	11.81
	11/28/07		7.05	NP	12.80
	01/09/08		7.18	NP	12.67
	02/13/08		7.92	NP	11.93
	03/19/08		7.40	NP	12.45
MW-21	02/28/07	17.70	6.78	NP	10.92
	03/28/07		6.64	NP	11.06
	04/30/07		7.63	NP	10.07
	05/23/07	İ	7.84	NP	9.86
	06/29/07		8.15	NP	9.55
Į	07/30/07		8.18	NP	9.52
İ	08/29/07		8.44	NP	9.26
	09/27/07		8.69	NP	9.01
1	10/25/07		8.16	NP	9.54
	11/28/07		8.16	NP	9.54
	01/09/08		6.94	NP	10.76
	02/13/08		7.18	NP	10.52
	03/19/08		7.92	NP	9.78
MW-22		17.55	6.87	NP	
IVI VV -22	02/28/07	17.55			10.68
	03/28/07		6.83	NP ND	10.72
	04/30/07		7.67	NP ND	9.88
	05/23/07		7.82	NP	9.73

		Top of Casing	Donath to	Duadwat	Groundwater
337 11 37 1	D (C II) (I	, -	Depth to Groundwater	Product	
Well Number	Date Collected	Elevationa	1	Thickness	Elevation
	0.6.10.0.10.77	(feet)	(feet)	(feet)	(feet)
MW-22, cont.	06/29/07		8.14	NP	9.41
	07/30/07		8.01	NP	9.54
	08/29/07		8.38	NP	9.17
	09/27/07		8.60	NP	8.95
	10/25/07		8.15	NP	9.40
	11/28/07		8.08	NP	9.47
	01/09/08		6.98	NP	10.57
	02/13/08		7.27	NP	10.28
	03/19/08		7.76	NP	9.79
MW-23	02/28/07	17.36	5.53	NP	11.83
	03/28/07		5.26	NP	12.10
	04/30/07		6.45	NP	10.91
	05/23/07		6.84	NP	10.52
	06/29/07		7.41	NP	9.95
	07/30/07		7.50	NP	9.86
	08/29/07		7.69	NP	9.67
	09/27/07		8.14	NP	9.22
	10/25/07		7.36	NP	10.00
	11/28/07		7.33	NP	10.03
	01/09/08		5.92	NP	11.44
	02/13/08		5.91	NP	11.45
	03/19/08		6.91	NP	10.45
MW-29	11/28/96	15.45	NM	NM	NM
	01/27/97		2.25	NP	13.20
	05/22/97		NM	NM	NM
	10/10/97		1.82	0.01	13.64*
	10/31/97		1.25	Film	14.20
	11/07/97		1.21	Film	14.24
	01/14/98		0.26	0.01	15.20*
	02/26/98		0.78	Film	14.67
	04/02/98		1.18	Film	14.27
	04/30/98		1.96	0.01	13.50*
	06/02/98		1.68	Film	13.77
	06/30/98		2.36	0.03	13.12*
	07/23/98		2.64	0.03	12.84*
	08/31/98		3.12	0.01	12.34*
	09/30/98		3.42	0.01	12.04*
	10/29/98		3.42	NP	12.03
	11/30/98		1.09	NP	14.36
	12/30/98		0.61	NP	14.84
	01/29/99		0.00	NP	>15.45
	02/25/99		0.00	NP	>15.45
	03/31/99	l	0.00	NP	>15.45
	04/30/99		1.08	NP	14.37
	06/02/99		2.02	Film	13.43
	06/30/99		2.26	NP	13.19
ľ	07/14/99		2.51	Film	12.94
	08/18/99		2.85	NP	12.60
			i i	NP NP	12.00
	09/15/99		3.18		
	10/28/99		3.42	NP	12.03

		Top of Casing	Depth to	Product	Groundwate
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
		(feet)	(feet)	(feet)	(feet)
MW-29, cont.	11/24/99		1.80	NP	13.65
	12/21/99		0.99	Film	14.46
	01/18/00		1.22	Film	14.23
	03/01/00		0.00	NP	>15.45
	03/30/00		0.41	Film	15.04
	04/26/00		1.21	Film	14.24
	05/31/00		2.18	Film	13.27
	06/29/00		2.56	NP	12.89
	07/25/00		2.80	NP	12.65
	08/15/00		3.06	NP	12.39
	09/12/00		3.10	NP	12.35
	10/17/00		3.22	NP	12.23
	11/14/00		2.78	NP	12.67
	12/29/00		2.61	NP	12.84
	01/19/01		2.79	NP	12.66
	02/27/01		2.42	NP	13.03
	03/26/01		2.26	NP	13.19
	04/30/01		1.80	NP	13.65
	05/28/01		2.71	NP	12.74
	06/25/01		2.69	NP	12.76
	07/23/01		2.98	NP	12.47
	08/27/01		2.97	NP	12.48
	09/23/01		3.51	NP	11.94
	10/25/01		3.29	NP	12.16
	11/29/01		0.73	NP	14.72
	12/29/01		0.21	NP	15.24
	01/31/02		0.27	Film	15.18
	02/28/02		0.60	NP	14.85
	03/28/02		0.48	Sheen	14.97
	04/24/02		1.41	NP	14.04
	05/30/02		2.26	NP	13.19
	06/27/02		2.53	NP	12.92
	07/30/02		2.90	NP	12.55
	08/29/02		3.21	NP	12.24
	09/27/02		3.49	NP	11.96
	10/31/02		3.74	NP	11.71
	11/26/02		3.33	NP	12.12
	12/26/02		2.37	NP	13.08
	01/30/03		0.72	NP	14.73
	02/27/03		1.74	Film	13.71
	03/31/03		1.12	Film	14.33
	04/28/03		1.33	Film	14.12
	05/29/03		2.28	NP	13.17
	06/25/03		2.70	NP	12.75
	07/30/03		3.12	Film	12.73
	08/28/03		3.12	NP	12.03
	08/28/03		3.42	NP NP	11.83
	10/29/03		2.11	NP NP	13.34
	10/29/03		1.38	NP NP	13.34
	01/09/04		0.51	NP NP	14.07 14.94
	01/09/04 01/27/04		0.51	Film	14.94 14.86

Table 1 Groundwater Elevation and NAPL Thickness Monitoring Results Port of Tacoma Former Milwaukie Railyard Site Tacoma, WA

		Top of Casing	Depth to	Product	Groundwater
Well Number	Date Collected	Elevation ^a	Groundwater	Thickness	Elevation
W CII INUIIIOCI	Date Conceied	(feet)	(feet)	(feet)	(feet)
MW-29, cont.	02/26/04	(1661)	0.61	Film	
W - 29, COIII.	03/29/04				14.84
	Į.		1.25	Film	14.20
	04/30/04		2.22	0.01	13.23
	05/25/04		2.60	0.02	12.87*
	06/30/04		2.88	NP	12.57
	07/28/04		3.23	NP	12.22
	08/25/04		3.11	NP	12.34
	09/29/04		3.10	Film	12.35
	10/28/04		2.82	NP	12.63
	11/23/04		3.05	NP	12.40
	12/30/04		9.36	NP	NA ^b
	01/31/05		9.06	0.01	NA ^b
	02/28/05		5.36	NP	NA ^b
	03/30/05		4.56	NP	NA ^b
	04/27/05		4.52	NP	NA ^b
	05/26/05		4.39	NP	NA ^b
	06/28/05	18.01	5.12	NP	12.89
	07/26/05		5.55	NP	12.46
	09/08/05		6.14	0.01	11.87
	09/30/05		6.30	Film	11.71
	10/28/05		6.38	Heavy Sheen	11.63
	11/30/05		4.98	Film	13.03
	12/28/05		3.92	NP	14.09
	01/27/06		3.61	Film	14.40
	02/27/06		4.22	Sheen	13.79
	03/28/06		4.57	Sheen	13.44
	04/25/06		4.74	NP	13.27
	05/30/06		4.91	NP	13.10
	06/27/06		5.22	Sheen	12.79
	07/24/06		5.54	NP	12.47
	08/30/06		6.00	0.01	12.01
	09/29/06		6.37	0.01	11.64
	10/31/06		6.48	Sheen	11.53
	11/30/06		4.35	NP	13.66
	12/28/06		3.41	NP	14.60
	01/31/07		3.99	NP	14.02
	02/28/07		3.72	NP	14.02
	03/28/07		3.72	NP	14.29
	03/28/07		4.49	NP	13.52
	05/23/07		4.76	NP	13.32
	06/29/07		5.55	Sheen	13.23
	07/30/07			Sheen	i i
	1		5.60	1	12.41
	08/29/07		5.96	Sheen	12.05
	09/27/07		6.20	Sheen	11.81
	10/25/07		4.32	NP	13.69
	11/28/07		5.50	NP	12.51
	01/09/08		4.45	NP	13.56
	02/13/08		4.55	NP	13.46
	03/19/08		4.89	NP	13.12

Table 1 Groundwater Elevation and NAPL Thickness Monitoring Results Port of Tacoma Former Milwaukie Railyard Site Tacoma, WA

Well Number	Date Collected	Top of Casing Elevation ^a (feet)	Depth to Groundwater (feet)	Product Thickness (feet)	Groundwater Elevation (feet)
OB-2	02/28/07	18.95	4.63	NP	14.32
	03/28/07		4.53	NP	14.42
	04/30/07		6.05	NP	12.90
	05/23/07		6.52	NP	12.43
	06/29/07		7.12	NP	11.83
	07/30/07		7.23	NP	11.72
	08/29/07		7.67	NP	11.28
	09/27/07		7.95	NP	11.00
	10/25/07		7.33	NP	11.62
	11/28/07		7.13	NP	11.82
	01/09/08		6.57	NP	12.38
	02/13/08		6.27	NP	12.68
	03/19/08		6.81	NP	12.14

Notes:

NA = not available.

NAPL = Nonaqueous phase liquid

NM = depths to groundwater and free product were not measured.

NP = free product was not detected in the well.

- = not sampled.

* = Groundwater elevation corrected for product thickness by the equation: TOC - (DTW)-[PT x 0.89]; where TOC = top of casing elevation, DTW = depth to groundwater, PT = product thickness, and 0.89 = specific gravity of product.

To minimize table length and still provide seasonal data, this table contains the last 13 months of groundwater monitoring data for wells that have never contained free product.

This table contains all of the monitoring data collected since November 1996 for wells that have contained product.

Observation well OB-N was removed in December 2004 during site development activities.

- ^a Top of casing elevations are based on 1997 survey results.
 - Top of casing elevations for CW-2, CW-6, and CW-7 were resurveyed on January 9, 2004.
- Top of casing elevations for CW-2, CW-3, CW-4, CW-5, CW-8, OB-S and MW-29 were resurveyed on June 10, 2005.
- b Casing has been altered due to site development work; groundwater elevations are not available.
- c Viscosity of product did not allow for accurate measurement of product thickness or groundwater depth.

Table 2 Summary of Goundwater Analytical Results Port of Tacoma Former Milwaukee Railyard Site Tacoma, Washington

Well	Date	NWTPH-Dx (Diesel)	NWTPH-Dx (Oil)	Benzene	Toluene	Ethylbenzene	Total Xyenes	Benzo(a)anthracene	Chrysene	Benzo(a)pyrene	Indeno(1,2,3-cd)pyrene	Dibenzo(a,h)anthracene	Benzo(b)fluoranthene	Benzo(k)fluoranthene	Naphthalene	pcBs³
Number	Sampled		(mg/L)		BIEX	(µg/L)				c-PA	Hs + Nap	hthalene	(µg/L)			PCBs (μg/L)
CW-1	7/28/04 1/10/08 3/20/08	2.93 0.48 2.4	2.27 <0.26 2.4									 				
CW-2	7/28/04 1/11/08 3/20/08	<0.118 <0.13 <0.13	<0.237 <0.26 <0.26		 				 			 				
CW-3	7/29/04 1/11/08 3/20/08	<0.12 0.46 0.23	<0.24 <0.26 <0.25	 								 				
CW-4	1/10/08 3/20/08	1.9 2.3	0.62 1.2													
CW-5	1/10/08 3/20/08	13 4.7	2.8 2.0													
CW-6	7/28/04 1/11/08 3/20/08	<0.119 <0.13 <0.13	<0.239 <0.26 <0.26	 			 	 		 						
CW-7	7/28/04 1/11/08 3/19/08	<0.12 <0.13 <0.13	<0.241 <0.26 <0.26			 					 	 				
CW-8	7/28/04 1/11008 3/20/08	8.46 0.62 0.93	1.29 0.31 <0.26			 					 	 				
CW-9	7/28/04 1/9/08 3/19/08	<0.121 <0.13 <0.13	<0.242 <0.26 <0.26				 	 			 	 				

Table 2
Summary of Goundwater Analytical Results
Port of Tacoma Former Milwaukee Railyard Site
Tacoma, Washington

Well	Date	NWTPH-Dx (Diesel)	NWTPH-Dx (Oil)	Benzene	Toluene	Ethylbenzene	Total Xyenes	Benzo(a)anthracene	Chrysene	Benzo(a)pyrene	Indeno(1,2,3-cd)pyrene	Dibenzo(a,h)anthracene	Benzo(b)fluoranthene	Benzo(k)fluoranthene	Naphthalene	PCBs ^a
Number	Sampled	TPH ((mg/L)		BTEX	(µg/L)				c-PA	Hs + Nap	hthalene	(μg/L)			PCBs (µg/L)
MW-7	7/29/04	<0.12	<0.239													
	1/10/08	0.33	0.52													
	3/19/08	<0.13	<0.26													
MW-21	7/29/04 1/11/08 3/21/08	<0.12 <0.13 <0.13	<0.24 <0.26 <0.26	 <1.0 <1.0	<1.0 <1.0	<1.0 <1.0	 <3.0 <3.0	<0.31 <0.30	 <0.21 <0.20	<0.21 <0.20	<0.31 <0.30	<0.31 <0.30	<0.42 <0.40	<0.31 <0.30	<2.1 <2.0	 <0.53 <0.51
MW-22	7/29/04 1/11/08 3/21/08	<0.12 0.13 <0.13	<0.241 <0.26 <0.25	<1.0 <1.0	<1.0 <1.0	<1.0 <1.0	<3.0 <3.0	<0.31 <0.31	<0.20 <0.20	<0.20 <0.20	<0.31 <0.31	<0.31 <0.31	<0.41 <0.41	<0.31 <0.31	<2.0 <2.0	<0.50 <0.50
MW-23	7/28/04 1/11/08 3/21/08	<0.124 <0.13 <0.13	<0.248 <0.25 <0.26	<1.0 <1.0	<1.0 <1.0	<1.0 <1.0	 <3.0 <3.0	<0.31 <0.31	<0.21 <0.20	<0.21 <0.20	<0.31 <0.31	<0.31 <0.31	<0.42 <0.41	 <0.31 0.31	 <2.1 <2.0	<0.50 <0.50
MW-29	7/29/04 1/10/08 3/19/08	0.453 <0.13 0.33	0.716 <0.25 0.66	 										 		

Notes:

BTEX = Benzene, toluene, ethylbenzene, and total xylenes (analyzed by EPA Method 8260B)

c - PAHs = Carcinogenic PAHs (analyzed by EPA Method 8270C)

NWTPH-Dx = Northwest test Method NWTPH-Dx

PAHs = Polynuclear aromatic hydrocarbons

PCBs = Polychlorinated biphenyls (analyzed by EPA Method 8082)

TPH = Total petroleum hydrocarbons (analyzed by Northwest Test Method NWTPH-Dx)

<1.0 = Constituent was not detected at or above the method reporting limit listed.

mg/L = Milligrams per liter (or parts per million)

μg/L = Micrograms per liter (or parts per billion)

--- = Not analyzed

^a Individual PCB arochlors were not detected at the method reporting limit listed.

Table 3
Soil core Physical Properties Data - Grain Size Analysis
Port of Tacoma Former Milwaukee Railyard Site
Tacoma, Washington

						Parti	cle Size Dat	a			
							Particle Size	e Distribution	n (% weight))	
							Sand Size				
Boring Number	Date Sampled	Sample Depth (feet bgs)	Mean Grain Size Description ^a	Mean Grain Size (mm)	Gravel	Coarse	Medium	Fine	Silt	Clay	Silt and Clay
SC-1	6/14/2008	8.9	fine sand	0.335	0.00	0.00	40.06	54.75	3.80	1.39	5.19
SC-1	6/14/2008	11.65	fine sand	0.215	0.00	0.00	15.20	75.59	7.24	1.97	9.21
SC-1	6/14/2008	12.2	fine sand	0.348	0.00	0.00	41.22	53.86	3.67	1.25	4.92
SC-2	6/14/2008	8.7	medium sand	1.061	2.43	27.92	40.33	19.71	b	b	9.61
SC-2	6/14/2008	10.7	coarse sand	0.41	33.14	3.00	13.11	39.43	b	b	11.32
SC-2	6/14/2008	16.1	medium sand	0.504	0.00	0.00	62.03	34.30	2.85	0.81	3.66
SC-2	6/14/2008	16.6	fine sand	0.389	0.00	0.00	45.91	49.51	3.43	1.15	4.58

Notes:

bgs = below ground surface

mm = millimeter

^a Based on mean from Trask.

^b Mechanical sieve does not differentiate silt/clay fractions.

Table 4 Soil Core Physical Properties Data - Permeability and Hydraulic Conductivity Port of Tacoma Former Milwaukee Railyard Site Tacoma, Washington

					2	5 psi Confining Stress ^a	
Boring Number	Date Sampled	Soil Core Identification	Sample Depth (feet bgs)	Sample Orientation	Specific Permeability to Air ^b (millidarcy)	Effective Permeability to Water ^{c, d} (millidarcy)	Hydraulic Conductivity ^{c, d} (cm/s)
SC-1	6/14/2008	SC-1-10.5'-13.0'	11.8	Horizontal	5,346	2,410	0.00221
SC-2	6/14/2008	SC-2-15.0'-17.5'	15.95	Horizontal	4,841	2,178	0.00198
SC-2	6/14/2008	SC-2-15.0'-17.5'	16.75	Horizontal	6,139	3,858	0.00353

Notes:

bgs = below ground surface cm/s = centimeters per second

psi = pounds per square inch

a Test Methods API 40/EPA 9100.

Specific = no pore fluids in place.

c Native state or effective = with as-received pore fluids in place.

Permeability to water and hydraulic conductivity measured at saturated conditions.

Table 5
Soil Core Physical Properties Data - Pore Fluid Saturations
Port of Tacoma Former Milwaukee Railyard Site
Tacoma, Washington

						Den	sity ^b	Poros	ity ^{b, c}	Pore Fluid S	aturation ^{b, d}
Boring Number	Date Sampled	Soil Core Identification	Sample Depth (feet bgs)	Sample Orientation	Moisture Content ^a (% weight)	Bulk (g/cc)	Grain (g/cc)	Total (% Vb)	Air Filled (% Vb)	Water (% Pv)	LNAPL (% Pv)
SC-1	6/14/2008	SC-1-8.0'-10.5'	8.9	Horizontal	25.8	1.49	2.70	44.8	5.7	79.1	8.1
SC-1	6/14/2008	SC-1-10.5'-13.0'	11.65	Horizontal	24.1	1.46	2.69	45.6	9.8	69.6	8.8
SC-1	6/14/2008	SC-1-10.5'-13.0'	12.2	Horizontal	16.0	1.50	2.69	44.3	20.2	51.5	2.9
SC-2	6/14/2008	SC-2-7.5'-10.0'	8.7	Horizontal	67.4	0.82	2.07	60.5	4.7	64.2	28.1
SC-2	6/14/2008	SC-2-10.0'-12.5'	10.7	Horizontal	10.4	2.00	2.70	26.1	4.8	57.0	24.6
SC-2	6/14/2008	SC-2-15.0'-17.5'	16.1	Horizontal	24.2	1.53	2.73	44.0	6.0	40.1	46.3
SC-2	6/14/2008	SC-2-15.0'-17.5'	16.6	Horizontal	27.9	1.46	2.70	45.9	4.0	46.0	45.3

Notes:

bgs = below ground surface

g/cc = grams per cubic centimeter

Pv = pore volume

Vb = bulk volume

a Test Methods API RP40/ASTM D2216.

b Test Method API RP 40.

^c Total porosity = no pore fluids in place, all interconnected pore channels. Air filled porisity = pore channels not occupied by pore fluids.

^d Water = 0.9996 g/cc, SC-1 hydrocarbon = 0.8803 g/cc, SC-2 hydrocarbon = 0.9498 g/cc.

Table 6 Soil Core Physical Properties Data - NAPL Mobility, Initial and Residual Saturations Port of Tacoma Former Milwaukee Railyard Site Tacoma, Washington

		ŧ								aturations ^{b, c} Pv)	
					Den	sity ^a		Initial Fluid	Saturations	After Centrifu	ge at 1000xG
Boring Number	Date Sampled	Soil Core Identification	Sample Depth (feet bgs)	Sample Orientation	Bulk (g/cc)	Grain (g/cc)	Total Porosity ^a (% Vb)	Water (Swi)	LNAPL (Soil)	Water (Srw)	LNAPL (Sor)
SC-1 Note: No visi	6/14/2008 ble NAPL pr	SC-1-10.5'-13.0' oduced. Produced v	11.9 vater slightly	Horizontal cloudy with	1.53 strong hyd	2.67	42.6 or.	79.7	2.0	11.1	2.0
SC-2 Note: Black N	6/14/2008 IAPL produc	SC-2-15.0'-17.5' ced. Produced clear	15.9 water.	Horizontal	1.51	2.68	43.7	63.9	23.9	11.4	9.3
SC-2 Note: Dark br	6/14/2008 own NAPL _I	SC-2-15.0'-17.5' produced. Produced	16.9 clear water	Horizontal	1.58	2.73	42	48.7	46.1	9.1	15.0

1000xG = one thousand times the force of gravity.

bgs = below ground surface

g/cc = grams per cubic centimeter

NAPL = Nonagueous phase liquid

Pv = pore volume

Soi = Initial NAPL saturation as received prior to centrifuging at 1000xG

Sor = Residual NAPL saturation after centrifuging at 1000xG

Srw = Residual water saturation after centrifuging at 1000xG

Swi = Initial water saturation as received prior to centrifuging at 1000xB

Vb = bulk volume

Test Method API RP40.

Test Method ASTM D425M, Dean-Stark.

Water = 0.9996 g/cc, SC-1 LNAPL = 0.8803 g/cc, SC-2 LNAPL = 0.9498 g/cc.

Table 7

NAPL and Groundwater Geochemical and Physical Properties

Port of Tacoma Former Milwaukee Railyard Site

Tacoma, Washington

Well Number	Date Sampled	Geochemical Characterization ^a	Density (gm/ml)	Viscosity (centipoise)	Surface Tension Air/Water (dynes/cm)	Surface Tension NAPL/Water (dynes/cm)	Interfacial Tension Air/NAPL (dynes/cm)	Temperature of Measurements
CW-4 (NAPL)	6/8/2007	Extremely weathered middle distillate and a heavier material				(dynido/din)		
CW-5 (NAPL)	6/7/2007	Extremely weathered middle distillate and a heavier material						
RW-2L3 (NAPL)		Extremely weathered middle distillate and perhaps a very small amount of heavier material	0.8803	8.34	58.4	7.1	24.6	60
RW-2L5 (NAPL)		Extremely weathered middle distillate and perhaps a very small amount of heavier material						
RW-5L3 (NAPL)		Extremely weathered middle distillate and a heavier material	0.9498	633.00	56.5	13.7	29.6	60
RW-5L3 (water extract)		Extremely weathered middle distillate and perhaps a small amount of heavier material						

Notes:

F = degrees fahrenheit

dynes/cm = dynes per centimeter

gm/ml = grams per milliliter

NAPL = nonaqueous phase liquid

--- = not tested

^a Based on hydrocarbon characterization using gas chromatography (GC) to characterize the composition of petroleum hydrocarbons and the degree of weathering.

PORT OF TACOMA

FORMER MILWAUKEE RAILYARD SITE TACOMA, WASHINGTON

File: N:\Cad\Drawings\Port of Tacoma\August 2008\Figure 1.awg Layout: Layout1 User: jascha.coddington Sep 04, 2008 — 10:54am

haw Shaw Environmental, Inc.

REV

PROJECT NO. 130415

SCALE

300

600 FEET

REV DATE

DATE OF ISSUE <u>8/08</u>

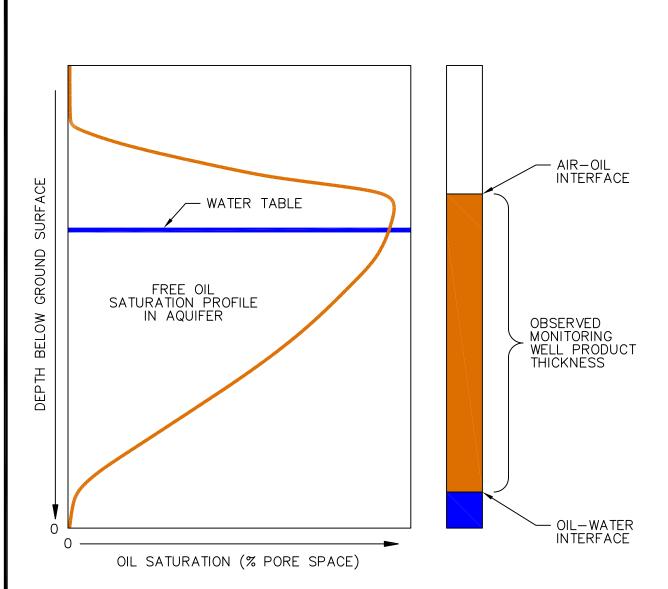
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APP BY

PROJECT NO. 111487



SOURCE: API, 2004, API INTERACTIVE LNAPL GUIDE, AUGUST 2004



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PR	OJECT NO.	
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FIGURE 3 CONCEPTUALIZATION OF NAPL WITHIN A MONITORING WELL

PORT OF TACOMA FORMER MILWAUKEE RAILYARD SITE TACOMA, WASHINGTON

111487

DATE OF ISSUE <u>8/08</u>

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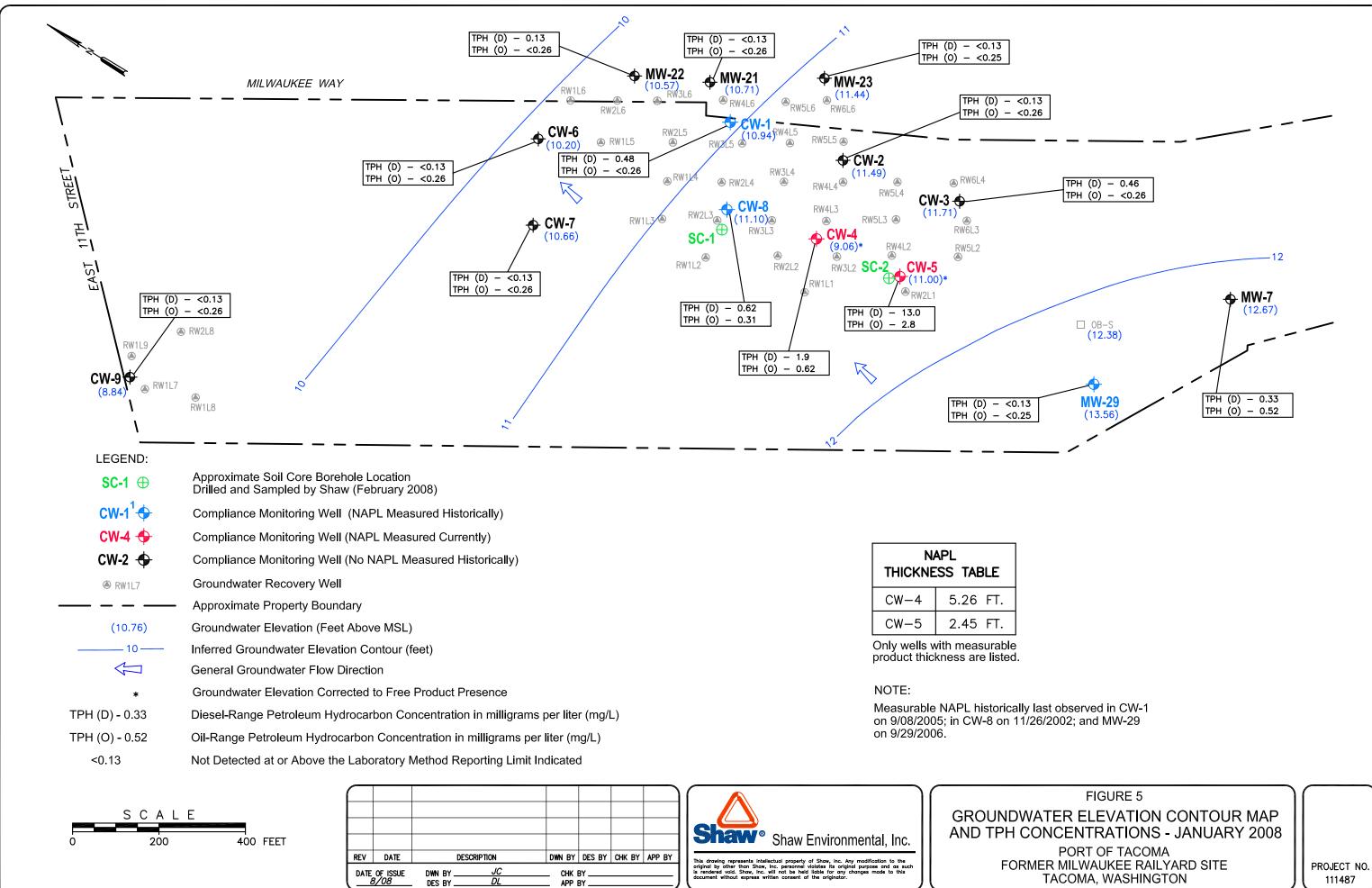
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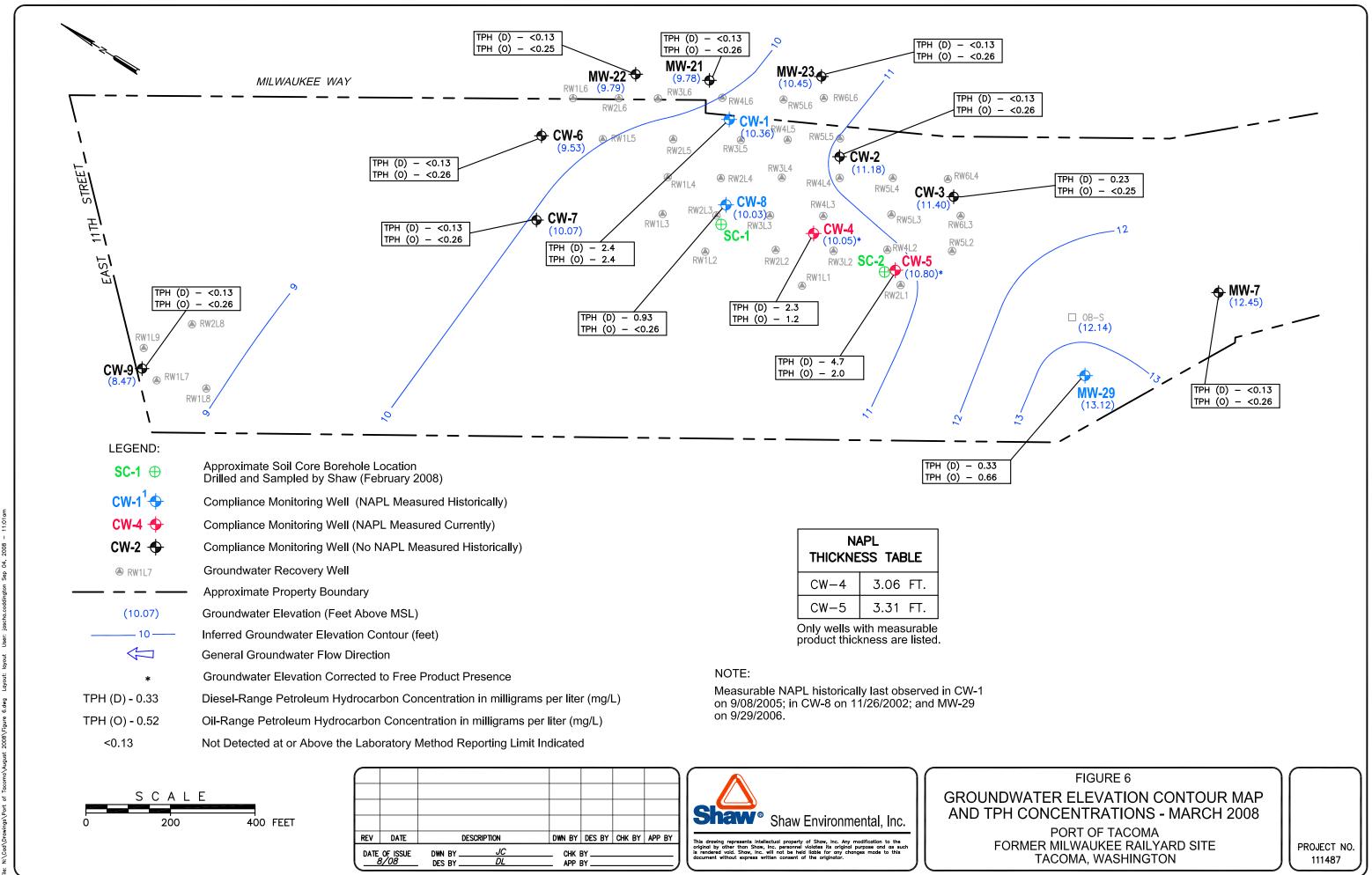
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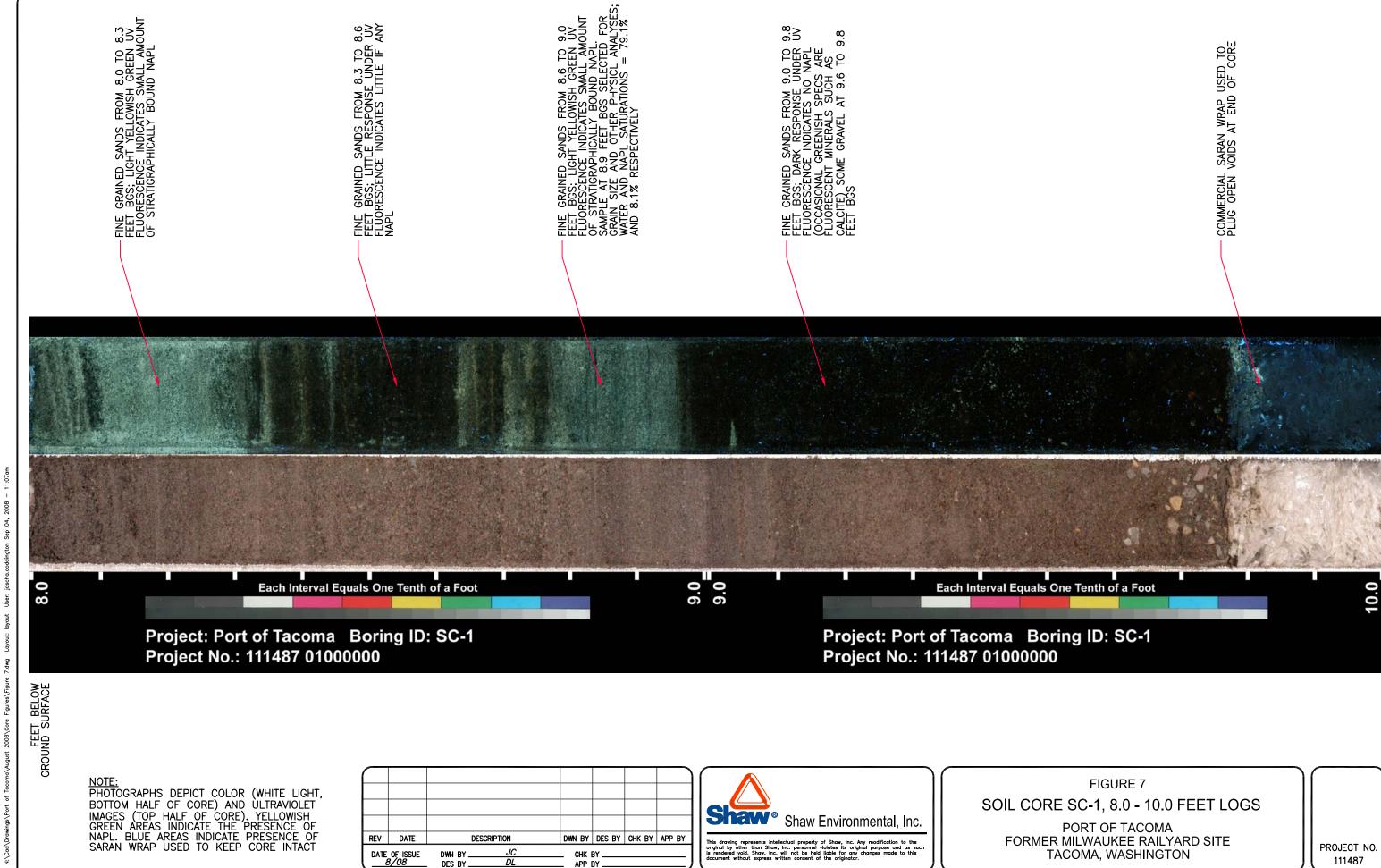
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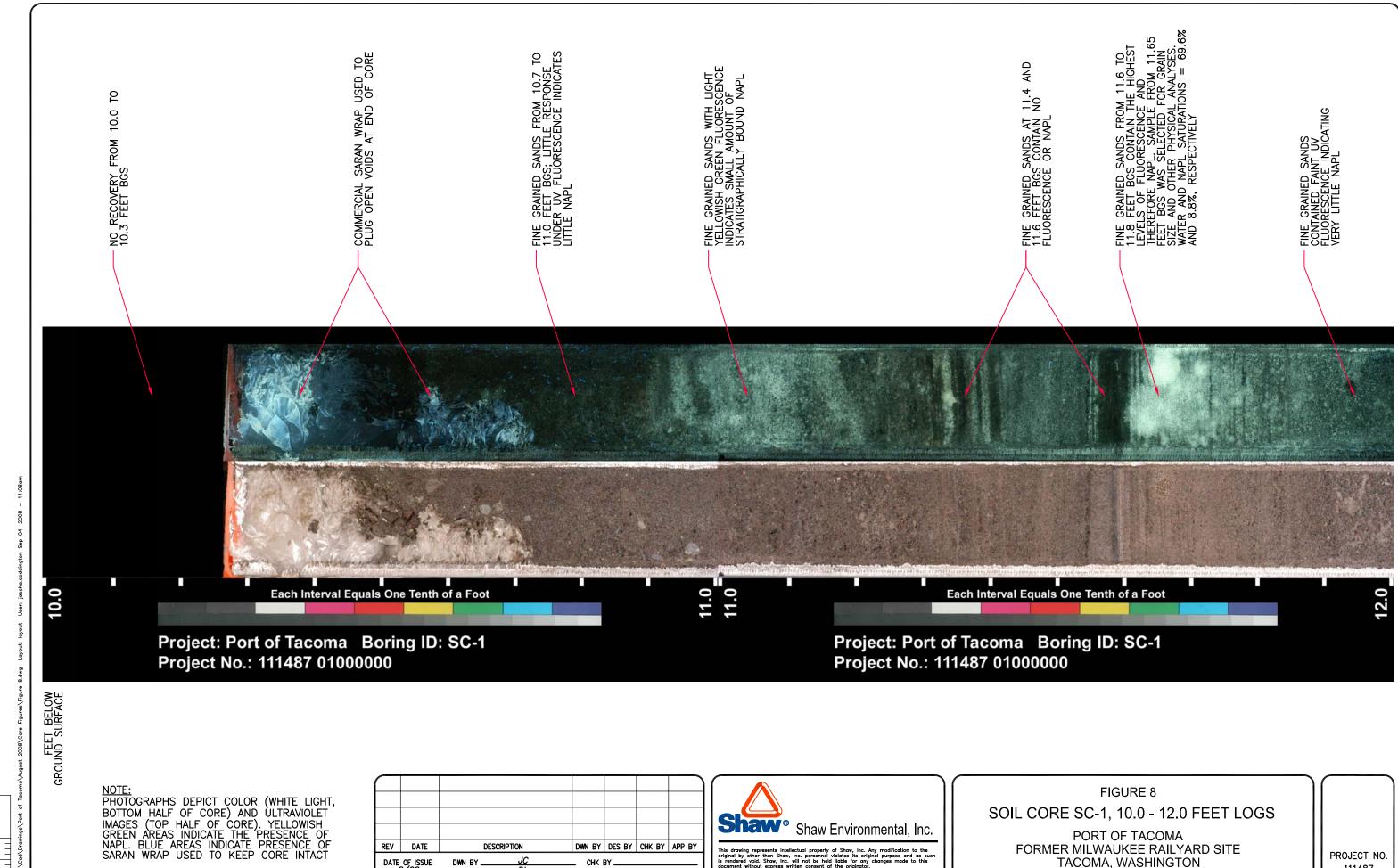
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TACOMA, WASHINGTON

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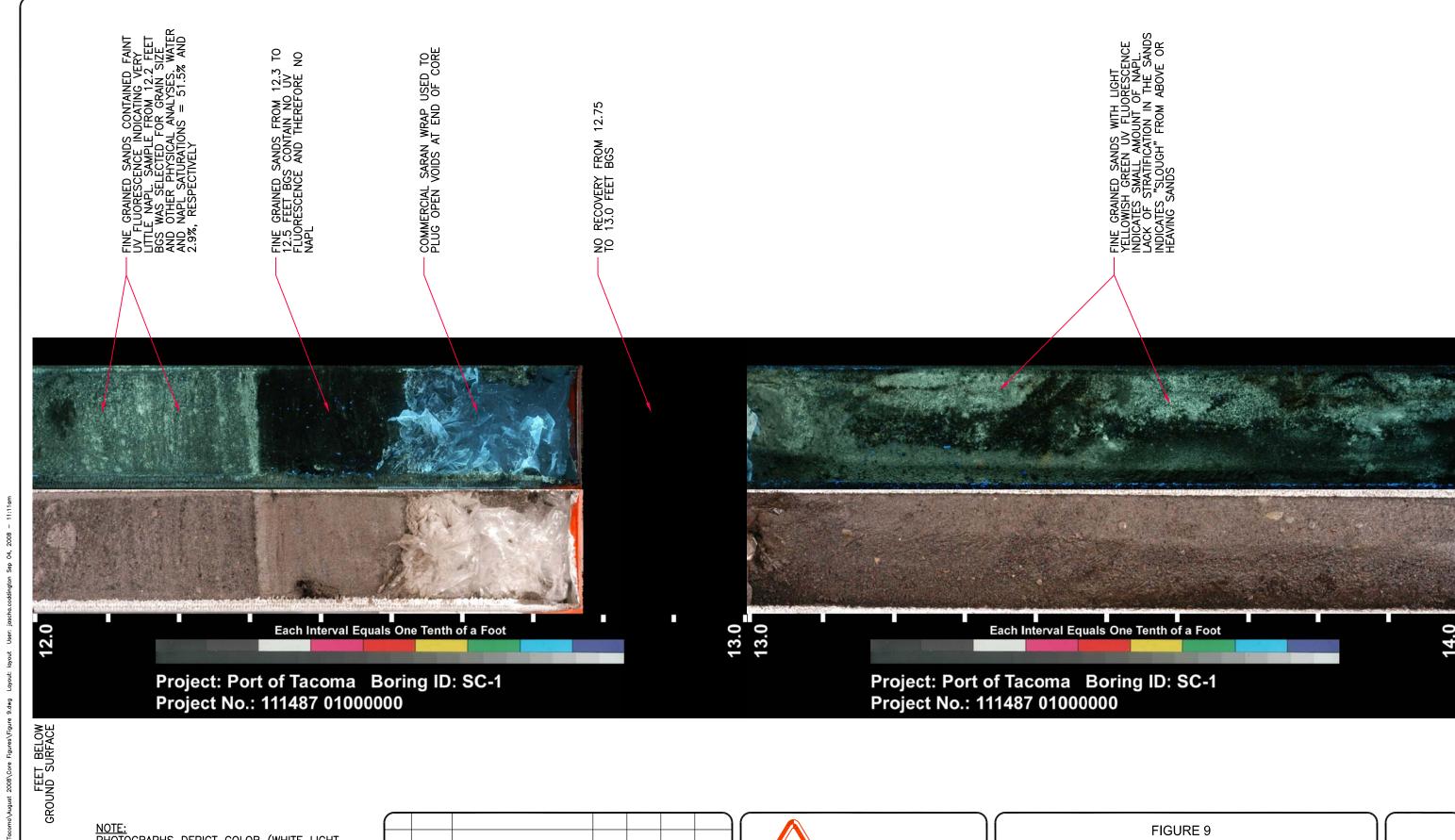
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NOTE:
PHOTOGRAPHS DEPICT COLOR (WHITE LIGHT,
BOTTOM HALF OF CORE) AND ULTRAVIOLET
IMAGES (TOP HALF OF CORE). YELLOWISH
GREEN AREAS INDICATE THE PRESENCE OF
NAPL. BLUE AREAS INDICATE PRESENCE OF
SARAN WRAP USED TO KEEP CORE INTACT

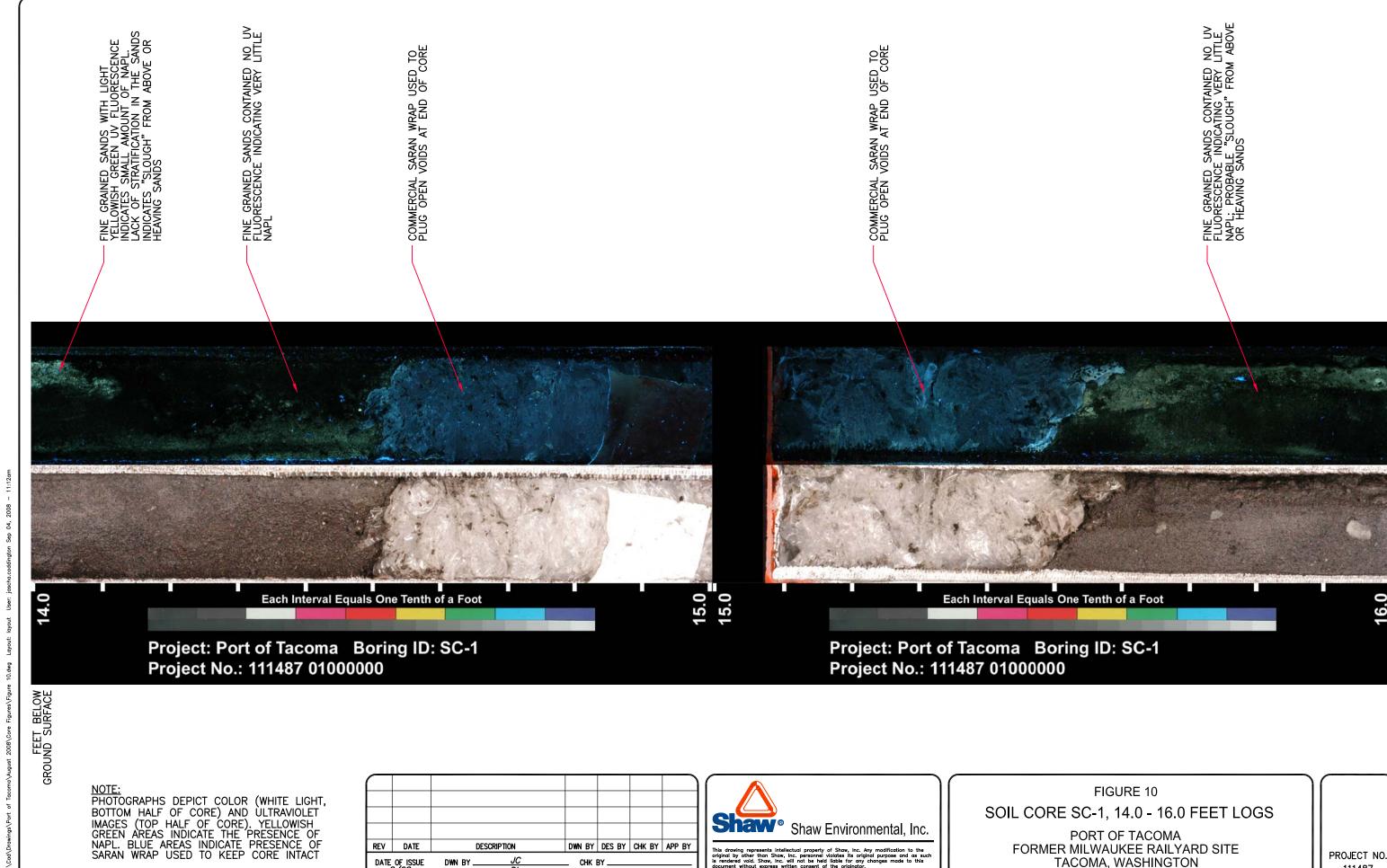
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SOIL CORE SC-1, 12.0 - 14.0 FEET LOGS PORT OF TACOMA

FORMER MILWAUKEE RAILYARD SITE TACOMA, WASHINGTON

PROJECT NO. 111487



REV DATE

DATE OF ISSUE 8/08

DESCRIPTION

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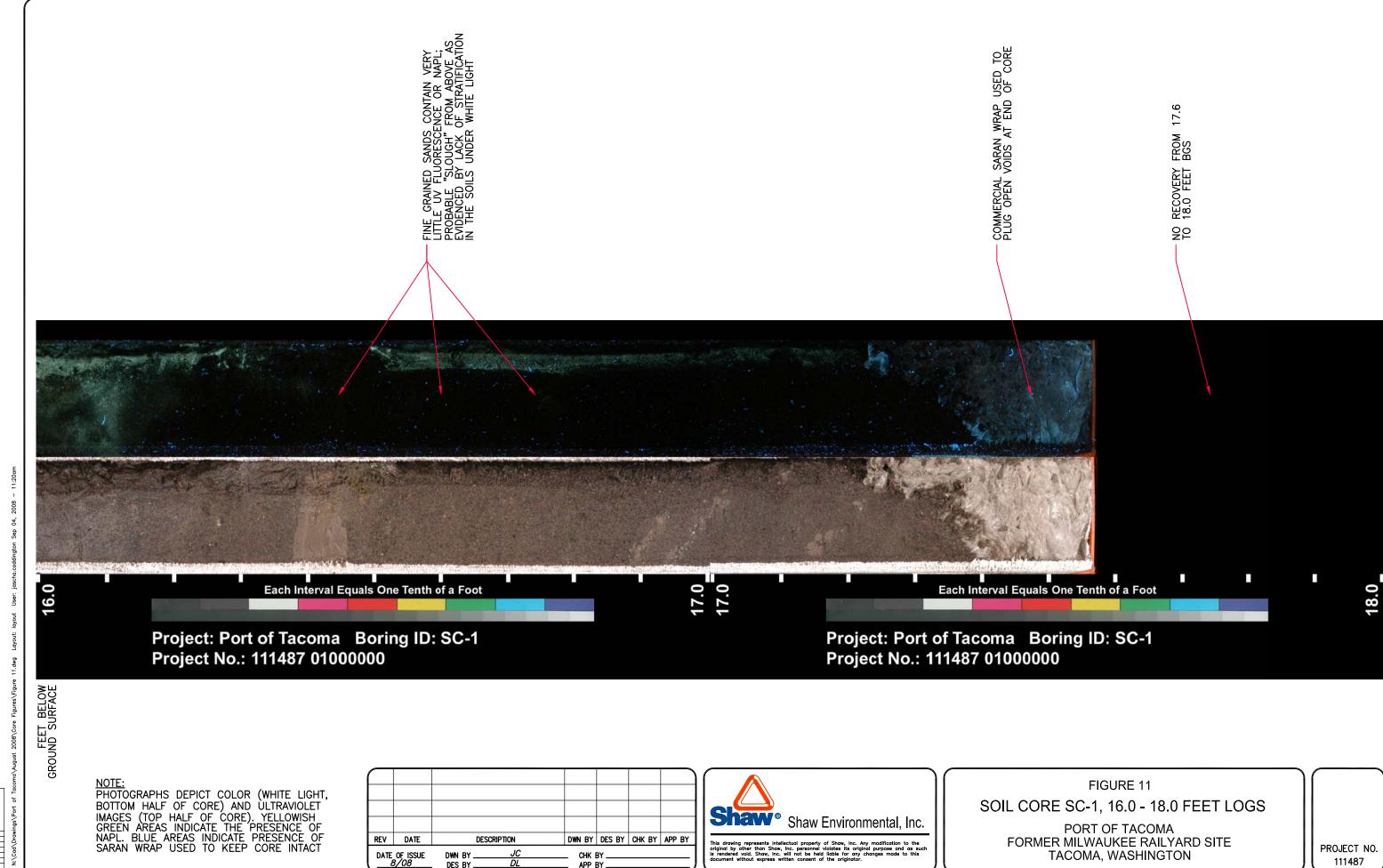
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PORT OF TACOMA FORMER MILWAUKEE RAILYARD SITE

TACOMA, WASHINGTON

PROJECT NO.

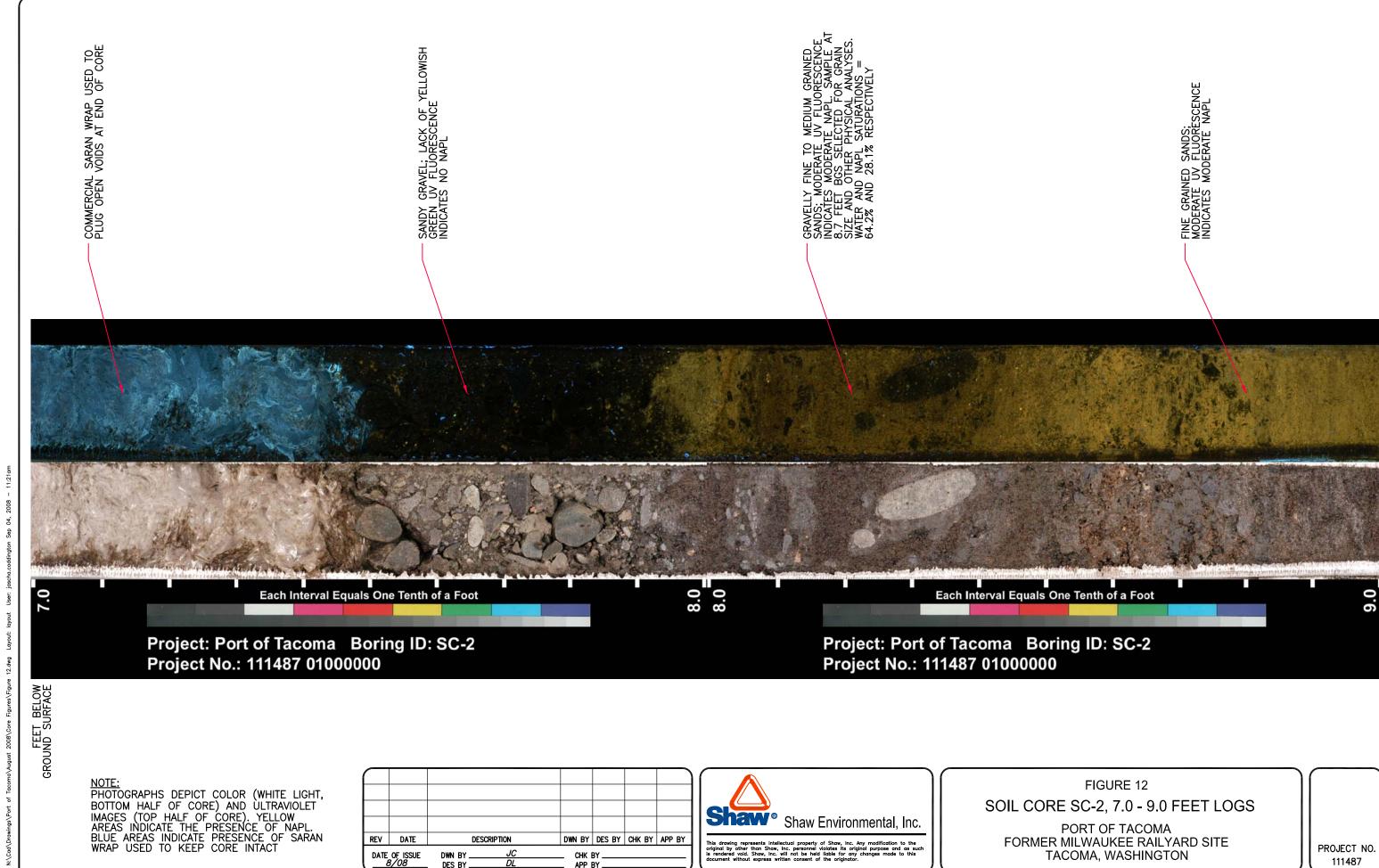
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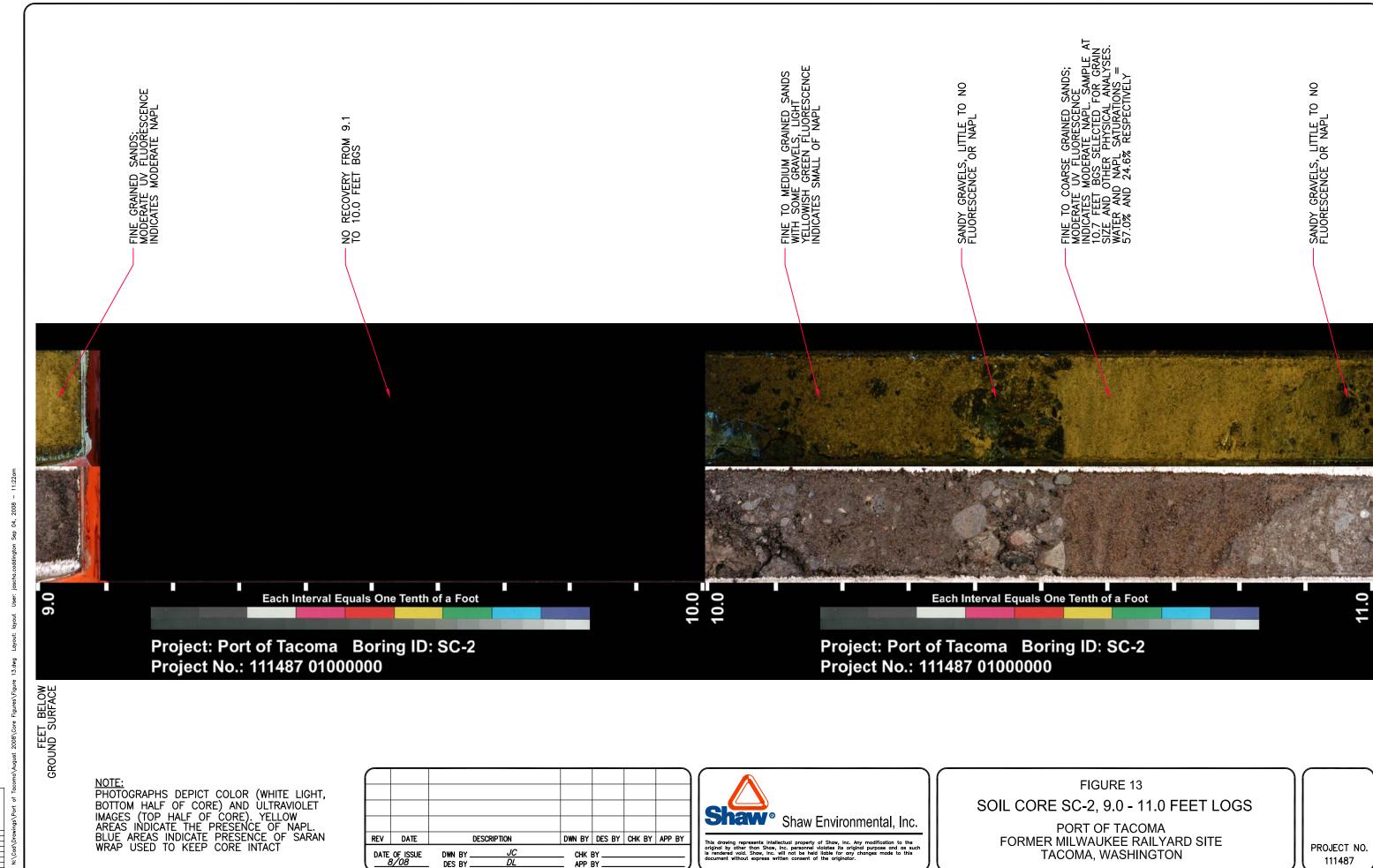
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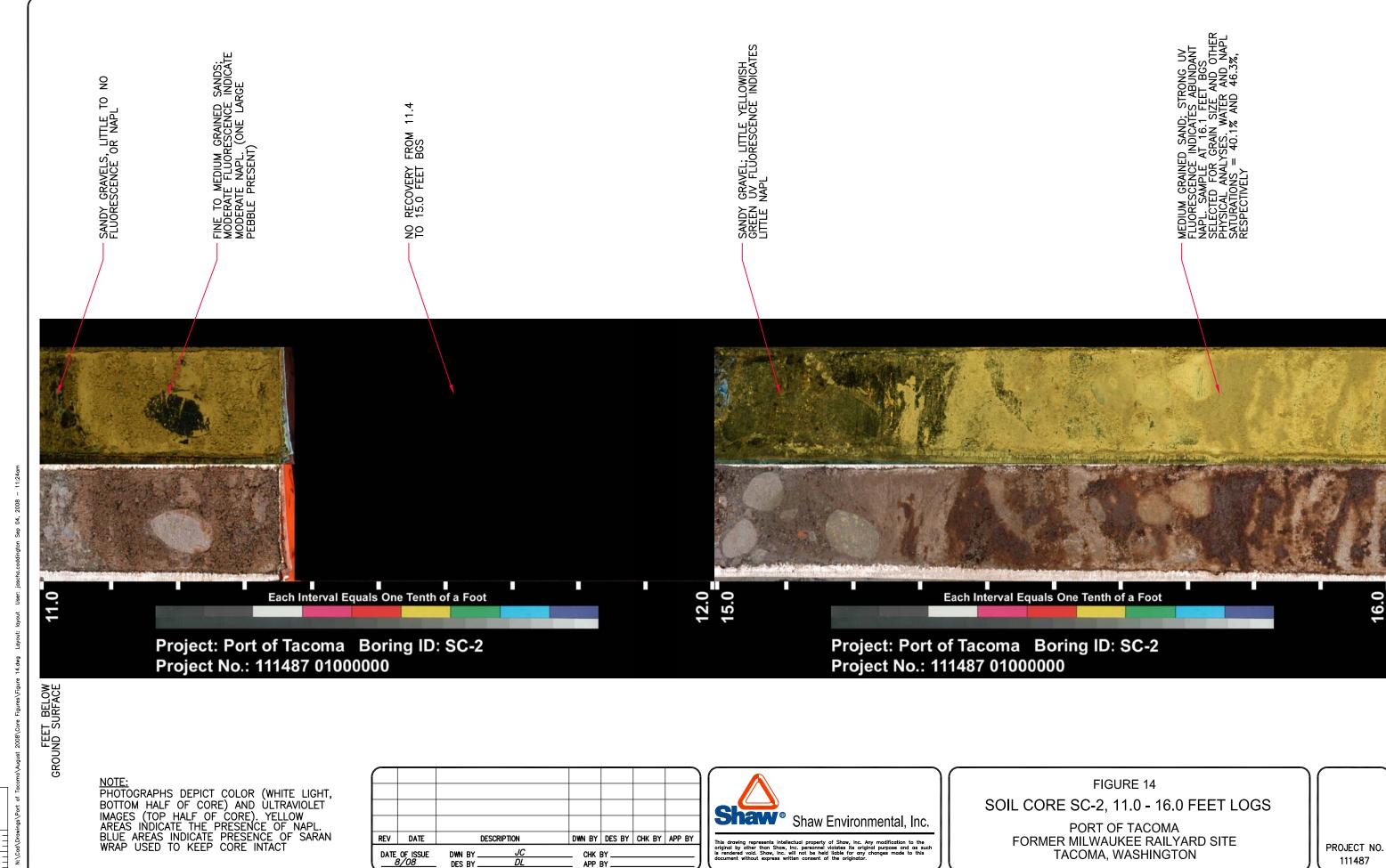
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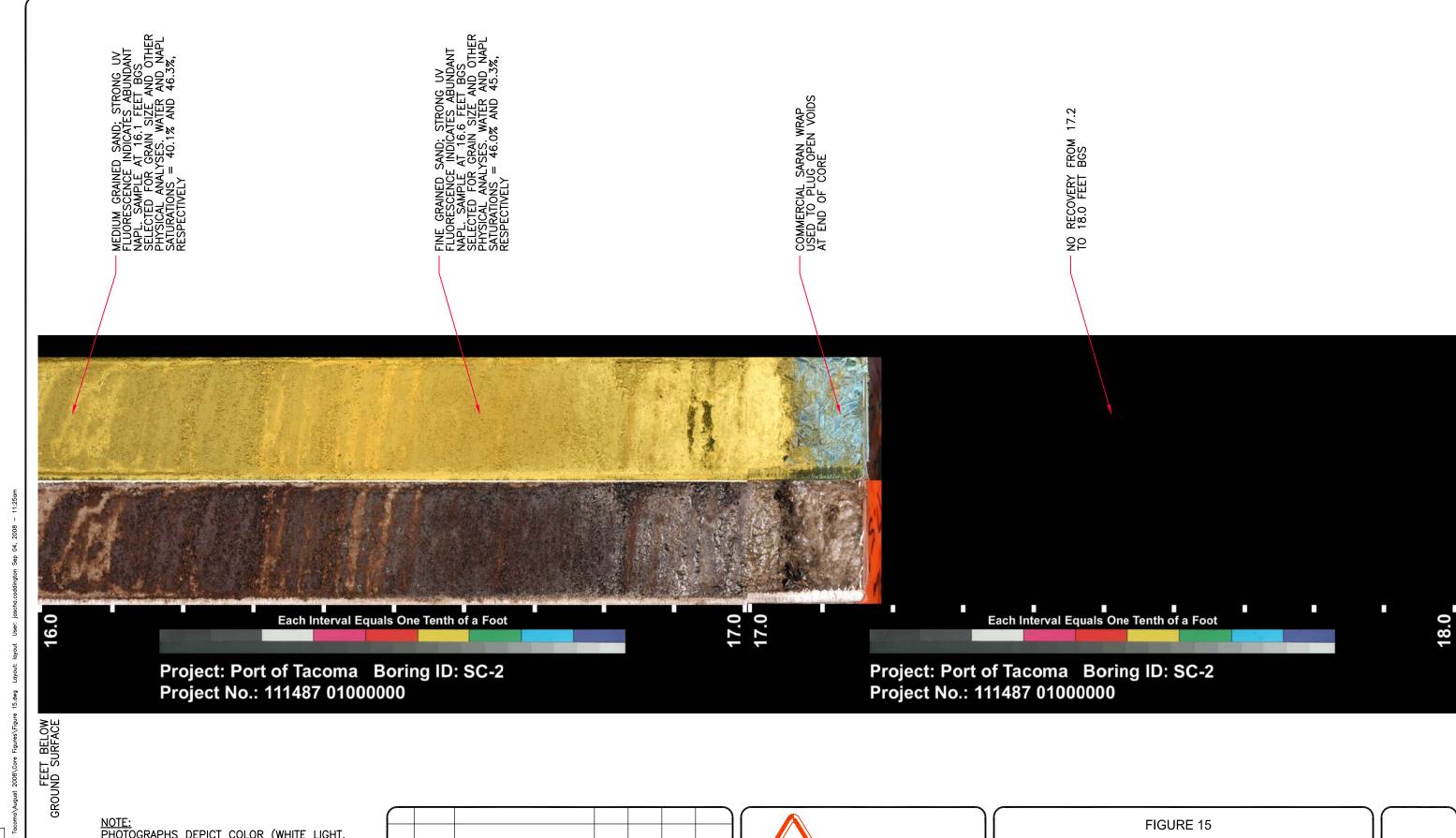


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1, 1/2, 0, |



NOTE:
PHOTOGRAPHS DEPICT COLOR (WHITE LIGHT,
BOTTOM HALF OF CORE) AND ULTRAVIOLET
IMAGES (TOP HALF OF CORE). YELLOW
AREAS INDICATE THE PRESENCE OF NAPL.
BLUE AREAS INDICATE PRESENCE OF SARAN
WRAP USED TO KEEP CORE INTACT

REV	DATE	DESCRIPTION	DWN BY	DES BY	CHK BA	ADD BY
	OF ISSUE 8/08	DWN BY JC DES BY DL		BY		

Shaw® Shaw Environmental, Inc.

SOIL CORE SC-2, 16.0 - 18.0 FEET LOGS

PORT OF TACOMA FORMER MILWAUKEE RAILYARD SITE TACOMA, WASHINGTON

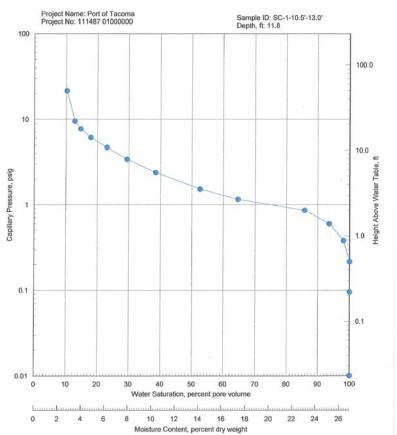
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SC-1 @ 11.8 feet BGS

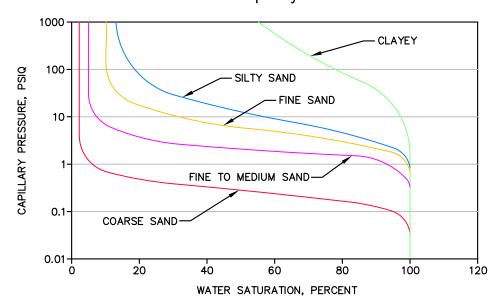


Shaw Environmental File No.: 38090

CAPILLARY PRESSURE
Centrifugal Method
r Displacing Water System - ASTM D6836



Characteristic Capillary Pressure Curves



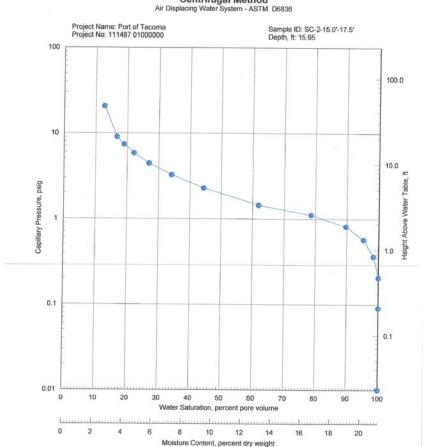
SOURCE: ADAPTED FROM BECKETT, G.D. AND HUNTLY, D., EVALUATING HYDROCARBON REMOVAL FROM SOURCE ZONES AND ITS EFFECT ON DISSOLVED PLUME LONGEVITY AND MAGNITUDE, API PUBLICATION NO. 4715, SEPTEMBER 2002

SC-2 @ 15.95 feet BGS

DTC





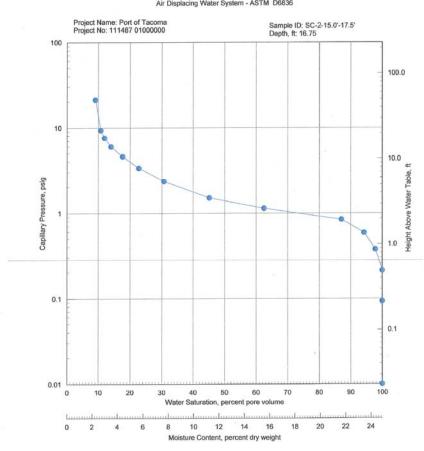


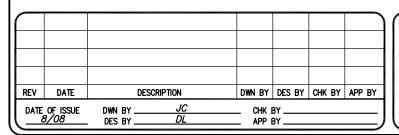
SC-2 @ 16.75 feet BGS

PTS

Shaw Environmental File No.: 38090

CAPILLARY PRESSURE
Centrifugal Method
Air Displacing Water System - ASTM D6836







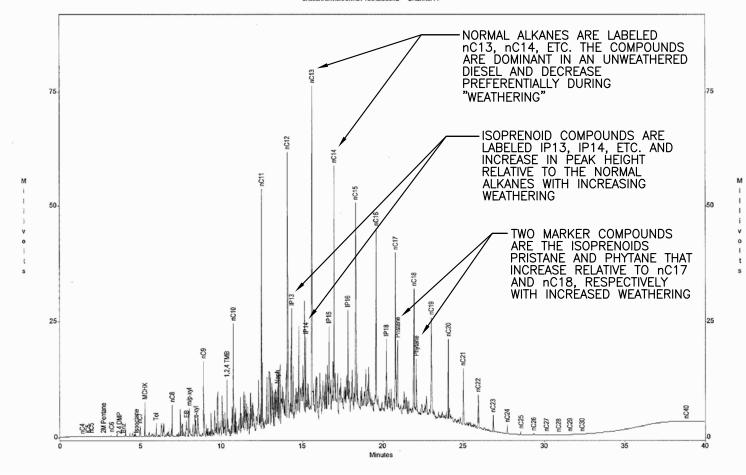
Diesel Standard

Torkelson Geochemistry, Inc.

Sample ID : Diesel

Acquired : Aug 20, 2007 16:18:02

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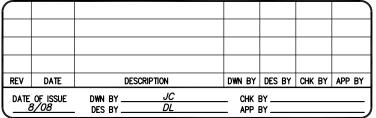




FIGURE 17
GAS CHROMATOGRAM OF NAPL
SAMPLE RW-2L3 AND LABORATORY
STANDARD FOR DIESEL
PORT OF TACOMA
FORMER MILWAUKEE RAILYARD SITE
TACOMA, WASHINGTON

PROJECT NO. 111487

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Appendix A Behavior of NAPL in the Subsurface

Introduction

A nonaqueous phase liquid results from the physical and chemical differences between liquid hydrocarbon and water, such that a physical interface exists between the two liquids. Hence, nonaqueous phase liquids act as a distinct fluid within the subsurface. Nonaqueous phase liquids, which are commonly referred to by the acronym "NAPL," have typically been divided into two general categories, light and dense. These terms describe the specific gravity or the density of the NAPL with respect to water. Light nonaqueous phase liquids, termed "L"NAPLs, have a specific gravity less than water and dense nonaqueous phase liquids, termed "D"NAPLs, have a specific gravity greater than water. Examples of LNAPLs include most fuels (gasoline, diesel, jet A, heating oil) and lubricants. DNAPLs include chlorinated solvents, creosote based wood-treating oils, coal tar wastes, and pesticides. Although many of the same principles and concerns apply to both LNAPLs and DNAPLs, the focus of this guide is on LNAPLs since these compounds comprise the most common type of contaminants at petroleum retail, storage, distribution and refining sites.

LNAPL contamination poses one of the most significant issues faced by the environmental industry. In particular, LNAPL contamination has typically been perceived as a significant environmental threat by the general public and the regulatory community, and as a result, LNAPL cleanup standards are generally very conservative. Technically, the remediation of LNAPL is difficult because it is significantly influenced by the physical character of the product, the nature of the soil conditions, and the hydrologic setting. LNAPL acts as a source of volatile and dissolved contaminants that may require remediation. Additionally, if sufficient volume exists, LNAPL may migrate posing significant environmental and legal concerns. To effectively manage the various and difficult aspects of LNAPL contamination, one must conceptually and technically understand the sometimes complex issues posed by LNAPL contamination. It is only through this understanding that appropriate management decisions can be made.

The management of LNAPL contaminated sites is a scientific and an engineering challenge. The focus of this primer is to present general fundamental concepts in understanding how subsurface processes and conditions influence the movement and retention of LNAPL in the subsurface. Specifically, these processes require knowledge of chemistry, geology, hydrology, soil science, biology, and engineering. The objective of this primer will be to provide general information on key factors to consider and recognize when managing a LNAPL site. This *NAPL Basics* primer will form the basis of more technical discussions presented in other primers.

LNAPL Movement in the Subsurface

Products typically produced, stored, and distributed include gasoline, middle distillates (diesel, kerosene), and heavy fuel and lubricating oils. These products vary in chemical composition and physical properties. The characteristics of these product types in conjunction with the hydrogeologic conditions at the site and the manner in which the product is released are the primary factors that influence the movement and distribution of LNAPL in the subsurface. When oil is accidentally released at the surface or from an underground pipe or storage tank, oil migrates vertically downward under the force of gravity. When the volume of the release is sufficient, the LNAPL will migrate through the unsaturated zone to the capillary fringe and water table (Figure 1). The increasing water content in the capillary fringe and the effects of buoyancy will impede the vertical movement of the LNAPL near the water table. As a result, the less dense oil will begin to migrate laterally along the water table. In general, the lateral oil migration will preferentially flow with the water table gradient. However, if the rate of downward vertical LNAPL movement from the surface is greater than the lateral migration, the oil will begin to mound vertically and oil flow may become radial. In addition, downward migration into the aquifer will increase displacing water from the aquifer pore space.

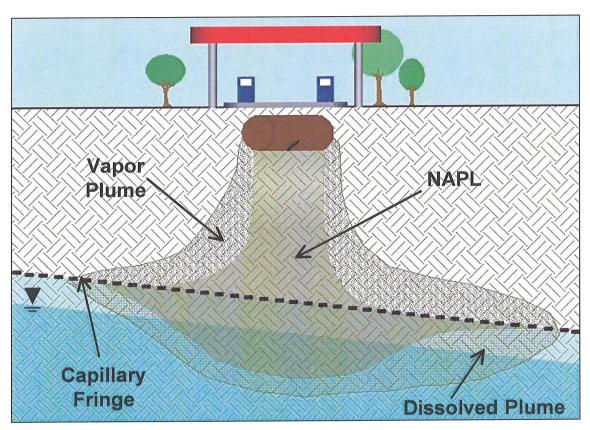


Figure 1. Representation of a LNAPL Release to the Subsurface.

In the aquifer, LNAPL coexists with water in the soil pores. The percentage of LNAPL filling the total pore space is termed the oil saturation. Due to the presence of water in the soil, LNAPL saturations are never 100 percent but may range from as little as 5 percent to over 70 percent (Figure 2). The percent saturation and distribution of LNAPL within the pore network will change over time as oil initially displaces water and is then subsequently displaced as water refills pore spaces when water levels rise.

In the past, a common misconception of the vertical distribution of free product at the water table was based on the idea that LNAPL occurs as a distinct lens in which the drainable pore space is completely saturated with LNAPL. This was often referred to as the "pancake layer" conceptualization (Figure 3). Under the pancake layer paradigm LNAPL saturations are 100 percent. This paradigm predicts large free oil volumes, high mobilities, and large recoverable volumes. Most importantly, the paradigm does not inherently consider soil and product properties as significant. An updated paradigm that is more representative of typical soil "multiphase" capillarity is referred to as the saturation which LNAPL conceptualization, in decreases continuously with depth (Figure 3). In the aguifer, LNAPL coexists in the soil pores with water over a given thickness. An examination of the impacted aguifer indicates that the LNAPL produces a profile with decreasing LNAPL saturation with depth. Specifically, the oil phase will displace the water by pushing into the pore spaces of the upper portions of the profile. With depth, the amount of oil pushing into the pores becomes less and less until at some depth no water is displaced and the pores remain completely filled with water (100 percent water saturation). Field studies have indicated that the multiphase conceptualization provides a good representation for both coarse and fine grained soils.

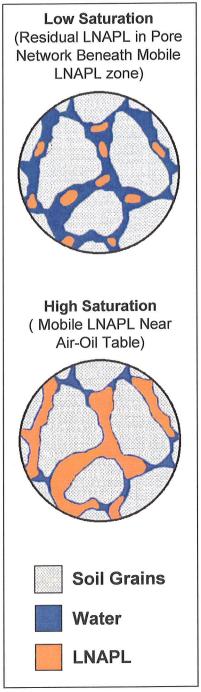


Figure 2. Representation of LNAPL Saturations in the Soil Pore Space.

LNAPL When a well penetrates saturated soil, oil and water will migrate into the well bore and reach equilibrium relative to the atmospheric conditions. As a result, a distinct layer of oil will develop in the well above the water. The thickness of oil in the well will reflect the thickness of the aquifer in which some amount of mobile oil saturation is present (Figure 4). The upper surface of the oil layer in the well is termed the air-oil interface and the lower surface of the oil is termed the oil-water interface. The actual water potentiometric level cannot physically measured in the well. This interface must be calculated using the density of the oil (ρ_0) , the elevation of the water-oil interface (Z_{ow}) , and the

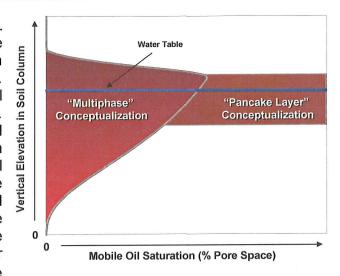


Figure 3. Conceptualization of LNAPL Vertical Distribution in Soil Profile.

LNAPL thickness measured in the well (H_o). The following equation is utilized in determining the theoretical air-water interface (Z_{aw}) in a well containing LNAPL.

$$Z_{ow} = Z_{ow} + (\rho_o H_o) \tag{1}$$

The volume of LNAPL per area of aquifer is primarily dependent upon the properties of the soil and the LNAPL. Since water remains in some of the pore spaces (for some soils, the majority of the pore spaces), the amount of oil in the formation is less than the monitoring well might suggest. In general, for a given observed well product thickness, the mobile LNAPL volume is greater for coarse-grained aquifer material than for soils composed of silt and clay. The importance of grain size is discussed in the primer *Soil*

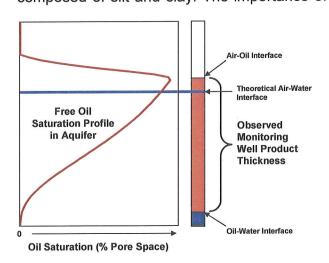


Figure 4. Conceptualization of LNAPL within a Monitoring Well.

Properties. Similarly, the volume of mobile oil will vary depending on product composition for a given observed well product thickness. The nature and properties of LNAPLs are discussed in the primer Product Types.

Due to capillary forces, some LNAPL is always retained in the soil pores as residual or immobile LNAPL. remaining "untrapped" LNAPL is mobile and may continue to migrate. As LNAPL moves within the subsurface, volume of mobile or "free" product decreases LNAPL continually as becomes trapped as isolated droplets network. within soil pore the particular, it becomes difficult for the oil to coalesce into a consistent plume of any significant thickness. Hence, LNAPL plumes, unless continually supplied from an on-going release, are "spatially self-limiting." This important concept distinguishes LNAPLs from dissolved and vapor plumes that may migrate significant distances.

Other Considerations

The dynamic and heterogeneous character of the subsurface influences LNAPL conditions. For example, slight differences in soil texture may promote preferential pathways within the aquifer, or conversely, may inhibit product migration causing LNAPL to pool as a stratigraphic trap. Similarly, fractures may create anisotropic conditions that cause product to flow in a direction not directly downgradient. In addition, LNAPL is significantly influenced by vertical fluctuations in the water table. These fluctuations enhance the development of residual LNAPL, which in many cases stabilizes the LNAPL movement. The primers on *Water Table Fluctuations* and *Heterogeneous Conditions* discuss how transient and spatial conditions may influence LNAPL migration.

In the subsurface, constituents composing the LNAPL will begin to transfer from the oil phase to the vapor and liquid phases (Figure 1). The transfer of volatile components to the soil air creates a vapor phase in the unsaturated zone above and adjacent to the LNAPL. Similarly, the transfer of soluble components to the water creates a dissolved plume in the saturated zone. The resulting vapor and dissolved plumes may readily migrate away from the LNAPL plume. In many cases, these plumes contain toxic components that may pose human health and environmental risks. As such, an understanding of the mass transfer between the LNAPL and vapor/water phases is important in managing LNAPL plumes. The concepts of dissolution and volatilization are discussed in the primers *Dissolution* and *Volatilization*.

Because LNAPL may act as a continuing source of dissolved and volatile contamination, numerous technologies have been developed to facilitate recovery of both mobile and residual product. The applicability of the technology depends upon site-specific hydrogeologic conditions, the nature and distribution of the LNAPL, and the remedial objectives. Although remediation techniques are continuing to improve, most technologies remain limited in removing LNAPL from the subsurface. In particular, the extraction of residual LNAPL is problematic and retention of approximately 40 to 70 percent of the LNAPL mass within the aquifer may occur. Furthermore, the application and operation of some technologies may increase the retention of LNAPL. As such, the proper selection and appropriate operation of corrective measures is very important and may significantly influence long-term conditions at a site. These concepts are discussed in greater detail in the primer *Remediation Technologies*.

Since the effectiveness of current remedial efforts for residual phase product is limited, LNAPL contaminated sites need to be managed in a different manner. Perspectives of producing rapid clean-ups to background conditions are impracticable and generally not possible. LNAPL contaminated sites must be understood in terms of risk and managed appropriately. Hence, understanding must be gained in terms of LNAPL plume stability and recoverability as well as a source of contaminant mass transfer to the water and vapor phases. It is from this technical basis that LNAPL risk can be determined.

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Appendix B Field and Laboratory Methods

APPENDIX B

Field and Laboratory Testing Methods

1.0 Introduction

This appendix describes the field and laboratory testing methods implemented for Tasks 1, 2, and 3 of the additional investigation performed at the former Milwaukee Railyard Site in Tacoma, Washington (referred to herein as the Site). The field and laboratory testing methods were performed in accordance with the Additional Studies Work Plan (Work Plan) dated October 15, 2007 (TechSolv, 2007b) that was approved by the Washington State Department of Ecology (Ecology). The locations of boreholes and wells referenced in this appendix are shown in Figure 2 of the report text.

2.0 Task 1 - Groundwater Monitoring

Task 1 consisted of groundwater monitoring. Groundwater analytical testing was performed in accordance with U.S. Environmental Protection Agency (EPA) or Ecology approved test methods.

Field Methods

Groundwater monitoring activities were performed in January and March 2008. Results of the January and March 2008 groundwater monitoring events were previously reported to Ecology in the January 2008 and April 2008 monthly progress reports (Shaw, 2008b and 2008c). The monthly progress reports included the field sampling data sheets (FSDSs) and laboratory analytical data reports; therefore, these documents are not included in this report. Similar field methods were employed for both monitoring events and are briefly described below.

For each groundwater monitoring event, Shaw personnel initially measured the depths to groundwater and any nonaqueous phase liquid (NAPL) in 14 monitoring wells (CW-1 through CW-8, MW-22, MW-21, MW-23, CW-9, MW-7, and MW-29) and piezometer OB-2. An electronic oil-water interface probe was used to collect the measurements.

After measuring the depths to groundwater and NAPL, Shaw personnel collected groundwater samples from the same 14 monitoring wells described above (i.e., CW-1 through CW-8, MW-22, MW-21, MW-23, CW-9, MW-7, and MW-29). Monitoring wells with NAPL (CW-4 and CW-5) were sampled by first removing NAPL from the wells. The following describes the procedures used to remove the NAPL:

• A diaphragm pump (an air compressor and generator were used to supply air to the diaphragm pump) was used to facilitate NAPL removal. Flex hose tubing fitted to the

diaphragm pump was lowered into the 2-inch diameter well casing to a depth of approximately 2 to 3 feet below the level of the NAPL-groundwater interface. The wells were purged of NAPL by pumping with the diaphragm pump until NAPL was no longer observed discharging from the tubing (i.e., all of the NAPL encountered was removed from each well). Less than 1 gallon of NAPL was recovered from each well.

- A 1-inch-diameter, polyvinyl chloride (PVC) pipe fitted with a PVC end cap was then quickly lowered into the well casing to approximately 2 to 3 feet above the bottom of the well casing before any NAPL could re-enter back into the well casing. The PVC pipe was held in place using a clamp. Approximately 0.5 gallon of deionized water was poured into the PVC pipe to create a positive head in the pipe. A ½-inch diameter, PVC rod was then used to push the end cap off of the PVC pipe (the end cap was attached to a string and removed from the well after sampling was completed).
- Groundwater purging and sampling was facilitated using the 1-inch-diameter PVC pipe and a peristaltic pump. At least three casing volumes of groundwater were purged from the monitoring wells well prior to additional purging and sampling using low-flow sampling procedures as described below.

The monitoring wells were purged and sampled using a peristaltic pump fitted with new, disposable polyethylene tubing replaced for each well. Low-flow purging and sampling techniques were used. The discharge stream was directed into a flow-through cell during well purging for direct measurement of field parameters including temperature, pH, specific conductance, dissolved oxygen (DO), and oxidation-reduction potential (redox). After field parameter measurements stabilized, the pumping rate was reduced and the discharge tubing was disconnected from the flow-through cell. Groundwater samples were then collected directly into laboratory-supplied sampling containers.

Testing Methods

The groundwater samples were submitted under chain-of-custody to TestAmerica in Tacoma, Washington, for analytical testing. All samples were analyzed for diesel- and oil-range TPH by Northwest Test Method NWTPH-Dx using silica gel cleanup before testing. Samples collected from offsite compliance wells MW-21, MW-22, and MW-23 were also analyzed for Ecology-recommended parameters related to diesel-range organics and heavy oils as specified in Table 830-1 of the MTCA cleanup regulations. The parameters analyzed included benzene, toluene, ethylbenzene, and total xylenes (BTEX) by EPA Method 8260B, carcinogenic polynuclear aromatic hydrocarbons (PAHs) and naphthalene by EPA Method 8270C, and polychlorinated biphenyls (PCBs) by EPA Method 8082.

3.0 Task 2 – Test Boring and Soil Characterization

Task 2 consisted of collecting NAPL and undisturbed soil core samples within the NAPL zone for physical characterization. The characterization methodology was conducted in accordance

with NAPL guidance provided by the American Petroleum Institute (API, Interactive NAPL Guide Release 2.0.4, and supporting API document *Methods for Determining Inputs to Environmental Petroleum Hydrocarbon Mobility and Recovery Models* [Sale, 2001]). Soil core physical testing was performed in accordance API and ASTM test methods.

Field Methods

On February 14, 2008, two boreholes (SC-1 and SC-2) were advanced to collect soil cores in the NAPL impacted area using a CME-75, hollow-stem auger drill rig operated by Cascade Drilling of Woodinville, Washington. At each borehole location, Cascade Drilling utilized an air knife vacuum excavation system to remove soil to a depth of 5 feet bgs to prevent the potential for damage to possible subsurface utilities. Initially, a pilot borehole was drilled at each location to collect soil samples for visual observation and logging purposes, and to determine the target depths of cores to be sent to the laboratory for physical analyses. The pilot boreholes for SC-1 and SC-2 were advanced to total depths of 21 and 20 feet below ground surface (bgs), respectively. Soil samples were collected continuously in 18-inch long split spoon samplers. Sample blow counts were recorded every 6 inches for qualitative assessment of soil density. Soils were classified according to the Unified Soil Classification System. Logs of the pilot boreholes are provided in Appendix C. Photographs of the sampling activities are provided in Appendix F.

Upon completion of each pilot borehole, a second borehole was advanced within 10 to 15 feet of the pilot boreholes to collect undisturbed soil cores from the NAPL zone (based on observations from the pilot boreholes) for laboratory testing. The coring depths were selected to include the first depth interval where evidence of NAPL was encountered, and to ensure coring from above and below the water table.

Soil core borehole SC-1 was sampled from 8 to 18 feet bgs, and soil core borehole SC-2 was sampled from 5 to 20 feet bgs. An Osterberg fixed-piston sampler was used to enhance sample recovery due to the loose, granular nature of the subsurface soils, and because it prevents soil from entering the sample tube before the sampling depth is reached. The Osterberg sampler contains an activated fixed piston contained in an outer thin-walled, 2.5-foot-long, Shelby Tube sampler. The sampler assembly was lowered to the desired depth, and then the inner sampler piston was withdrawn while the outer thin-walled tube was forced into the sample interval for undisturbed soil collection. The method develops suction within the Shelby Tube sampler thereby minimizing the potential for soil loss through the open end of the sampler.

Soil core sampling using the Osterberg sampler did not allow for sample observation or logging in the field because the sampler was immediately sealed for shipment to the laboratory. Geologic logging of the cores was subsequently possible based on the core photographs and laboratory testing results. Records of the soil core recovery are provided in the borehole logs.

Sample recoveries from ranged from 60 to 85 percent for individual sample intervals. Core recovery was mostly complete in borehole SC-1. SC-2 had no recovery from sample intervals 5.0 to 7.5 feet, 9.1 to 10.0, 11.4 to 15.0 feet, and 17.5 to 20 feet bgs. Some of the poor recovery was due to sampler damage by occasional gravels and boulders within the formation (see Photos 2 and 3, Appendix E), and due to the inherent loose and granular nature of the soils.

Soil core handling and preservation were performed in accordance with recommendations by the testing laboratory (PTS Laboratories) and guidance described by the American Petroleum Institute (Sale, 2001) specifically for collection and handling of soil cores for NAPL investigation. Portions of the Shelby Tube that did not retain the soil cores were filled with commercially available Saran wrap to minimize core movement (and does not interact with the NAPL). Plastic end caps (provided with the Shelby Tubes) were then placed over the ends of the tubes and taped securely to the tubes. The soil cores were labeled with the sampler number, depth interval, arrows to show top and bottom, and were immediately placed in coolers frozen with dry ice so that migration of NAPL or groundwater within the cores was minimized and fluid saturation was preserved. The samples remained frozen with dry ice during storage and transportation to the laboratory.

Testing Methods

The soil cores were submitted under chain-of-custody to PTS Laboratories in Santa Fe Springs, California, for physical testing. The soil cores were tested to evaluate the porous media physical parameters that affect NAPL distribution and recoverability from the subsurface. The testing methods were conducted in accordance with ASTM and API methods specified in *Methods for Determining Inputs to Environmental Petroleum Hydrocarbon Mobility and Recovery Models* (Sale, 2001).

Soil core physical testing was performed in a phased and iterative approach. Initially, the cores were photographed using standard white light and also ultraviolet light to record NAPL fluorescence and visually assess NAPL distribution. Based on assessment of the core photographs (as discussed in the following sections), discreet soil samples were selected from core borehole SC-1 at 8.9, 11.65, and 12.2 feet bgs, and from core borehole SC-2 at 8.7, 10.7, 16.1, and 16.6 feet bgs) for testing of grain size analysis and pore fluid saturation properties. The testing included the following parameters:

- Grain (particle) size analysis by ASTM Method D422/D4464M
- Moisture content by API RP Method 40 and ASTM Method D42216
- Bulk and grain density, total and air-filled porosity, and water and NAPL pore fluid saturations by APR RP Method 40.

Results of the pore fluid saturation testing were used to select additional samples for testing of air/water drainage capillarity properties and NAPL mobility properties. Composite sample intervals were then collected from core borehole SC-1 at 11.7 to 11.9 feet bgs, and from core borehole SC-2 at 15.9 to 16.1 and 16.7 to 16.9 feet bgs. The testing included the following parameters:

- Intrinsic Permaeability (Water): Includes specific permeability to water (intrinsic permeability) and hydraulic conductivity, by API RP40 and ASTM D5084.
- Intrinsic Permeability (NAPL): Includes specific permeability to NAPL (intrinsic permeability), by API RP40.
- Drainage Capillary Pressure Data (centrifugal method; air/water). Includes initial and residual fluid saturations, final water production vs. capillary pressure, effective (total) porosity, bulk density, air permeability and hydraulic conductivity, by API 40, ASTM D425M, and EPA 9100.
- NAPL Mobility (centrifugal method): Apply centrifugal force of 1000 times gravity for one hour to demonstrate product mobility. Includes initial and residual pore fluid saturations, porosity and bulk density, by ASTM D425.

A copy of the laboratory test data prepared by PTS Laboratories (excluding soil core photographs), and a copy of the chain-of-custody form, are provided in Appendix D.

4.0 Task 3 - NAPL Chemical and Physical Characterization

Task 3 consisted of collecting light nonaqueous phase liquid (NAPL) and groundwater samples for chemical and/or physical characterization. Chemical testing (hydrocarbon fingerprinting analysis) was performed by capillary gas chromatography (GC) methods, and physical testing was performed in accordance with American Society for Testing and Materials [ASTM] methods.

Field Methods

NAPL samples were collected on June 7 and 8, 2007, from five recovery or compliance wells (RW2L3, RW5L3, RW2L5, CW-4, and CW-5). The samples were collected using a peristaltic pump. NAPL was pumped directly into laboratory-supplied containers that were immediately placed in coolers chilled with ice. The samples remained chilled with ice during storage and transportation to the laboratory. Groundwater samples were also collected concurrently from these wells and submitted to the laboratory testing for physical analyses (e.g., NAPL/water interfacial tension); however, only selected groundwater samples were analyzed.

Testing Methods

The NAPL and groundwater samples were submitted under chain-of-custody to Torkelson Geochemistry, Inc. of Tulsa, Oklahoma, for chemical testing. The NAPL samples were initially analyzed for hydrocarbon characterization using capillary GC to characterize the range in composition of petroleum hydrocarbons that are present and the degree of weathering. These data were then used to select NAPL samples (and the associated groundwater samples) from two recovery wells, RW2L3 and RW5L3, for physical testing. The samples from these wells were selected for analyses based on the GC results that indicated they represented end-members in the range of NAPL compositions (i.e., diesel and heavy lube oil-range hydrocarbons). The NAPL physical tests included the following:

- NAPL Density (ASTM Method 4052)
- NAPL Viscosity (Brookfield DV-II instrumentation)
- Surface Tension of Air/Water (ASTM Method D971)
- Surface Tension of Air/NAPL (ASTM Method D971)
- Interfacial Tension of NAPL/Water (ASTM Method D971)

One of the groundwater samples, collected from RW-5L3, was also analyzed by GC to evaluate the potential for the NAPL to partition to groundwater.

A copy of the NAPL evaluation report, including the GC chromatograms and NAPL physical properties test data, prepared by Torkelson Geochemistry, Inc., and a copy of the chain-of-custody form, are provided Appendix F.

Appendix C

Borehole Logs for SC-1 and SC-2



Drilling Log

SC-1

Soil Boring (P

(Pilot Borehole)
Page: 1 of 1

Location Tacoma, Washing Surface Elev. NA Top of Casing NA Screen: Dia NA Casing: Dia NA Fill Material Bentonite Chip Drill Co. Cascade Drilling Driller Checked By	Total Hole Depti Water Level Initi Length NA Length NA Los Metho Log By David I	Owner Port of Tacoma Proj. No. 111487 1 21.0 ft. North East al 9.5 ft. Static NA Diameter Type/Size NA Type NA Rig/Core CME-75 d Hollow Stem Auger Lamadrid (Shaw) Date 2/14/08 Permit # NA Description	COMMENTS Used 18-inch-long, stainless steel Dames and Moore split-spoon sampler. Depth to groundwater and LNAPL measured at 10.00 and 9.65 feet below top of casing, repectively, in nearby well CW-4 on 2/13/08.
Depth (ft.) PID (ppm) Sample ID % Recovery	Blow Count Recovery Graphic Log USCS Class.	(Color, Texture, Structure) Geologic Descriptions are Based on the US	ocs.
- 8 - PH-1-2 100% - PH-1-3 100% - PH-1-5 100% - PH-1-6 100% - PH-1-6 100% - PH-1-8 100% - PH-1-8 100% - PH-1-9 100% - PH-1-9 100% - PH-1-100	6 14 18 SP 10 11 SP 11 S	Utilized Air Knife to 6' below ground surface for utility @ 0-0.5': Asphalt @ 0.5-6': FILL, gravelly sand with some fines (SW), 3-inch diameter, some concrete chucnks, very wet in trace fines, predominantly very fine grained, some per 1-inch diameter, no hydrocarbon odor. @ 7': Becomes gray-brown, faint hydrocarbon odor. @ 8.5': 1/2-inch thick lens of slightly silty fine sand (@ 9.3': Same as at 7', becomes wet then quickly sattern hydrocarbon odor, grades to brown-gray. Hydrocarbon odor decreases with depth (moderate) @ 11.8': 1-inch thick sandy silt lens. @ 12': Becomes loose to medium dense. Rare pebbles, still predominantly very fine sand but fine and medium sand. Sheen on saturated soil, becomes medium dense. @ 17-17.4': Slightly silty, very fine to fine sand (SM) silt approximately 10 to 15 percent. @ 17.4': Very find to medium sand (SP), gray-brown dense, trace fines, some pebbles. @ 18.8' Sharp contact with very fine to fine sand (SF) medium dense, no fines, trace to some medium grain slightly finer grained with depth. Total depth = 21 feet bgs; Groundwater encountered Backfilled with bentonite chips.	some cobbles to upper 2' to 3'. moist , medium dense, ebbles, rare gravel to 5'. SM) turated with depth, odor). greater precentage of 5'. , brown-gray, saturated 6, saturated, medium 6'), gray, saturated, ned sand, becomes



Drilling Log

Soil Boring

SC-1 (Soil Core Borehole) Page: 1 of 1

Project Former Milwaukee RR S Location Tacoma, Washington Surface Elev. NA To Top of Casing NA W. Screen: Dia NA Le Casing: Dia NA Le Fill Material Bentonite Chips Drill Co. Cascade Drilling Driller Loc Checked By	COMMENTS SC-1 (Pilot Borehole) drilled adjacent to SC-1 (Core Borehole). Use Osterberg Shelby Tube system to collect soil croes. Core borehole log based on soil core photograph interpretation and grain size analysis.		
Depth (ft.) PID (ppm) Sample ID % Recovery Blow Count Recovery	Graphic Log USCS Class.	Description (Color, Texture, Structure) Geologic Descriptions are Based on the U	scs.
- 0	SP SPG ON SP SP	Air Knife Excavation to 5 feet below ground surface. Auger drill out to 8 feet below ground surface. UPPER SAND ② 8': Fine to medium sand (SP), gray, wet, trace to laminae, minor to moderate LNAPL from 8.0' to 8.3', to 12.3'. ② 9.6': scattered pebbles and gravels. Some layers with apparent fine to coarse sand (SW, 12.1') 12.3' - 12.5': Apparent silty fine sand or fine sandy silty fine sand or fine sandy silty fine to medium sand (SP), gray, wet, scattere stratification not apparent, minor LNAPL at 13.0' to 1 potential sloughing soils based on lack of stratification. Total Depth = 18' Backfilled with bentonite chips	10 percent fines, fine 8.6', to 9.0' and 11.0' 11.2' to 11.3', 11.9' to ill layer. d pebbles and gravels, 4.0', and 15.5' to 16.2',

Drilling Log

SC-2 Soil Boring (Pilot Borehole)
Page: 1 of 1

Project	Former Milwauk	ee RR S	Site		Owner Po	ort of Tacoma		COMMENTS
Location	Used 18-inch-long, stainless steel Dames and Moore							
Surface F	ev NA	Tot	tal Hole D)epth	20.0 ft. N	lorth	No. <u>111487</u> East	split-spoon sampler. Depth to groundwater and LNAPL
Ton of Car	sing NA	Wa	ter Level	Initia	NA S	tatic NA	Diameter	measured at 11.87 and 7.65 feet below top of casing, repectively,
Screen: D	ia NA	Ler	noth NA	4	Т	vpe/Size NA		in nearby well CW-5 on 2/13/08.
Casina: Di	o NA	l or	nath NA	4	T	vpe NA		
Casing. Di	Bentonite C	Chips	1911		Rig/Core	CME-75		
Deill Co	Cascade Drillir	na	D/I	ethor	Hollow Stem Auge	er		
Driller		100	By Da	vid L	amadrid (Shaw) Dat	e 2/14/08 F	Permit # NA	
Chacked F	av.		, , , , , , , , , , , , , , , , , , , ,	- 1	icense No.			
CITEOREGI	1 1							
	□ § □ ∑	is di	ပ္	Class.			Description	
Depth (ft.)	PID (ppm)	S 800	Graphic Log	SC		(Color	, Texture, Structure)	
	PID (ppm) Sample ID % Recovery	Blow Count Recovery	Ō	USCS			iptions are Based on the U	SCS.
- 0 -					Utilized Air Kn	ife to 5' below gro	und surface for utilit	y clearance.
_			SOCI		0 0-0.4': Asp	halt		
			DOS		@ 0.4-5.3' : FI	LL, slightly silty sa	and (SM), fine to me	ter, rare wood chunks.
- 2 -			NO S		moist, some g	laveis, some con	nes to o-mon diamo	tor, rare week erraras
			2007					
- 4 -								
			(600)					
	PH-2-1	6		SM	@ 5.3' : 1.5-in	ch thick lens of wo	ood chip debris.	
- 6 -	100%	6		SM	O S S' TIPPE	R SAND slightly	silty, fine to medium	sand (SM),
	PH-2-2	4 7		SM ML	brown-gray, we hydrocarbon o	ct, loose to modic	m dense, silt 5 to 15	percent, strong
- 8 -	100%	7	2 20 20	SM	@ 6' 15" thic	k sandy gravel la	ver, wet, sheen	
	PH-2-3	7			@ 6.2' : Same	as 5.5', minor gra	evel to 1-inch diame	ter, slightly more silt
	100%	12		SM	73 increasing with	depth.		
- 10 -	PH-2-4	9			@ 7': Slightly	sandy silt (ML), g	reen-gray, wet, loos 5-15 percent, strong	e to medium dense,
80	35%	9		GW	78' Slight	ly silty yen fine to	medium sand (SM), brown-gray, wet, silt 5 !!!
12 1	PH-2-5	7			II to 15 percent.	predominantly fin	e grained sand, som	ne rounded fine gravel,
5 - 12 -	55%	8		SP	strong hydroca	arbon odor.		NADI residue some
T.G.	PH-2-6	5			@ 9' : silt incre	eases to 10 to 20	percent, abundant L	NAPL residue, some
SHAWIT.G	100%	9		SP	010.5' : Fine 1	to coarse sandy g	ravel (GW), gray, sa	aturated, medium
	PH-2-7	9			dense, rounde	d gravel to 1-inch	daimeter some fine	s, abundant
Rev. 12/20/06 FORMER MILWAKEE RR SITE.GPJ	65%	16			LNAPL/sheen,	rare wood chunk	and (SP) saturated	gray, medium dense,
16 -	PH-2-8 65%	5 7			trace fines, mo	oderate LNAPL re	sidue.	, 51,
RR -	105%	7 8		SP	Slightly more s	silt with depth.		
4KE	PH-2-9 100%	9			@ 16.7' : trace	fines, pockets of	LNAPL.	8
18 -	10070	11						
ER -	PH-2-10	9		SP	@ 19' : moder	ate LNAPL residu	le.	
20 -	85%	9	· / · · · · · ·	SP	Total depth = 3	20' bas: [NAPL e	ncountered at appro	ximately 9' bgs.
2					Backfilled with	bentonite chips.		
20/0						-		
2 - 22 -								
Rev								
			tan'i mandata					
- 24 –			A THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN C					
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DRILLING LOG Rev: 12/20/06 FORMER MILWAKEE RR SITE.GPJ SHAW IT.GDT 5/29/08

Drilling Log

Soil Boring

SC-2

(Soil Core Borehole)
Page: 1 of 1

Location	Total Hole I Water Level Length	Depth I Initia A A ethod avid L	Owner	COMMENTS SC-1 (Pilot Borehole) drilled adjacent to SC-1 (Core Borehole) for lithology. Use Osterberg Shelby Tube system to collect soil croes. Core borehole log based on soil core photograph interpretation and grain size analysis.
Depth (ft.) (ft.) PID · (PDm.) Sample ID % Recovery	Blow Count Recovery Graphic Log	USCS Class.	(Color, Texture, Structure) Geologic Descriptions are Based on the US	scs.
- 0 - - - - - - - - -	55	GW SW SW W/SV	Air Knife Excavation to 5 feet below ground surface. @ 5' - 7.5' : No Recovery @ 7.9' : Fine to coarse sand (SW), wet, 5 to 10 percupt to 1.5 inch diameter, minor LNAPL. @ 8.8' : Fine to medium sand, wet, trace fines, no given to 1.0' : No recovery @ 10' : Interbedded sandy gravel (GW) and gravelly 3' thick, wet, little or no LNAPL in gravel layers, modulates. @ 11.4' - 15.0' : No recovery @ 15.3' : Fine to coarse sand (SW) wet, some gravels diameter, very minor LNAPL. @ 15.3' : Fine to medium sand (SW), wet, minor peb decreasing with depth, trace fines, abundant LNAPL @ 17.1' - 20' : No recovery Total Depth = 20' Backfilled with bentonite chips	el (GW), wet, no LNAPL. ent fines, few gravels ravel, minor LNAPL. sand (SW), beds up to derate LNAPL in sandy up to 1.5 inch

Appendix D PTS Laboratories Soil Core Test Data



May 20, 2008

David Lamadrid Shaw Environmental 10300 S.W. Nimbus Avenue, Suite B Portland, Oregon 97223

Re:

PTS File No: 38090

Port of Tacoma 111487 01000000

Dear Mr. Lamadrid:

Please find enclosed report for Physical Properties analyses conducted upon cores received from your Port of Tacoma; 111487 01000000 project. All analyses were performed by applicable ASTM, EPA, or API methodologies. An electronic version of the report has previously been sent to your attention via the internet. The samples are currently in storage and will be retained for thirty days past completion of testing at no charge. Please note that the samples will be disposed of at that time. You may contact me regarding storage, disposal, or return of the samples.

PTS Laboratories appreciates the opportunity to be of service. If you have any questions or require additional information, please give me a call at (562) 907-3607.

Sincerely,

PTS Laboratories

Michael Mark Brady, P.G.

Project Manager

Encl.

PTS Laboratories

Project Name: Port of Tacoma Project Number: 111487 01000000

TEST PROGRAM

PTS File No: 38090 Client: Shaw Environmental

_										·		4
		Notes	Bulk Fluid Keep core frozen									
Fluid	Properties	Package	Bulk Fluid								0	
Calculate	۸G	Params.	Calc.								0	
Free	Product	Mobility	Hor. 1.5"		11.7-11.9					15.9-16.1, 16.7-16.9	3	
A/W Drng.	Capillarity	Pkg.	Hor. 1"		11.7-11.9					15.9-16.1, 16.7-16.9	3	
Grain	Size	Anal.	Grab	×	x, x			×	×	×,×	_	
Pore Fluid	Saturation	Package	Hor. 1.5"	6'8	11.65, 12.2			8.7	10.7	16.1, 16.6		
Slab and	Core	Photo	1/4:3/4	2	က	2	က	က	2	3	18	
Core	Recovery	ff.	Plugs:	1.8	2	1.5	1.9	1.55	1.35	2.05	12.15	
	Depth	ij		8-10.5	10.5-13	13-15.5	15.5-18	7.5-10	10-12.5	15-17.5	7 cores	A omena
	CORE ID		Hor. 1.5"	SC-1-8'-10.5'	SC-1-10.5'-13.0'	SC-1-13.0'-15.5'	SC-1-15.5'-18.0'	SC-2-7.5'-10.0'	SC-2-10.0'-12.5'	SC-2-15.0'-17.5'	TOTALS:	1 oborotony Took Dangaron Motor

Laboratory Test Program Notes
Copy Larry Roberts/TechSolv Consulting on all project communication per D. Lamadrid/Shaw Environmental. Sample locations to be selected by Shaw Environmental and TechSolv personnel from core photography.

Take Grain Size Analysis samples from adjacent to Pore Fluid Saturation Pkg. sample locations. Take Free Product Mobility from adjacent to A/W Drng. Capillarity Pkg. sample locations.

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CHAIN OF CUSTODY RECORD

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#Od	TURNAROUND TIME 24 HOURS S DAYS 48 HOURS NORMAL 72 HOURS COMMENTS SAMPLE INTEGRITY (CHECK): INTACT ON ICE PTS QUOTE NO. PTS FILE: COMMENTS	Coordinate Teams,	of Techsonu	\$53 402-8277	· 9					ED BY		TIME
										4. RECEIVED	COMPANY	DATE
'SIS REQUEST	POROSITY: EFFECTIVE, ASTM D425M SPECIFIC GRAVITY, ASTM D854 BULK DENSITY (DRY), API RP40 or ASTM D2937 HYDRAULIC CONDUCTIVITY, EPA9100, API RP40, D5084 TOC: WALKLEY-BLACK TOC: WALKLEY-BLACK									D BY		$\mathcal{O}\mathcal{G}\mathcal{U}$ Date time date
ANALYSIS	HYDRAULIC CONDUCTIVITY PACKAGE PORE FLUID SATURATIONS PACKAGE CAPILLARITY PACKAGE PHOTOLOG: CORE PHOTOGRAPHY PHOTOLOG: CORE PHOTOGRAPHY PHOTOLOG: TOTAL, ASTM D2216									3. RELINQUISHED	COMPANY	40 рате
	NUMBER OF SAMPLES SOIL PROPERTIES PACKAGE											TIME 094C
	ZIP CODE ZIP CODE PHONE NUMBER (SOS) 603-100/ EAX NUMBER SITE E DEPTH, FT	8,0-10,5	1/4/08 1250 10,5-13,0	13.0-155	1/14/08 1305 115,5-18,0				*	EIVED BY/	NNV C	DATE 1-08
	Perdle (S)	24 24 N	7. N.	77 78 78					,	A P. RECEIVED BY	COMPANY (DATE
276	Ster A DATE	8/14/08/124S	\$114108	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		 				1	Fast	. 5
COMPANY	Shaw Environmental ADDRESS 1030 SU NIMBER PROJECT MANAGER PROJECT NAME ON OF COMMENTAL PROJECT NUMBER 487 O 100000 O 603)6 STE LOCATION SAMPLE ID NUMBER DATE TIME DI	150-1-8/4/0.5	150-1-10,5-13,0	VSC-1-13,0-15,5 1/1/08 1255 13,0-15	18c-1-155-18,0'					1. RELINQUISHED BY	COMPANY ENVIONMENTED	DATE TIME 1/15/08 11:00

PTS Laboratories, Inc. • 8100 Secura Way • Santa Fe Springs, CA 90670 • Phone (562) 907-3607 • Fax (562) 907-3610 PTS GeoLabs. Inc. • 4342 W. 12th St. • Houston. TX 77055 • Phone (713) 680-2291 • Fax (713) 680-0763

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CHAIN OF CUSTODY RECORD

PAGE E

with Lang Roberts SAMPLE INTEGRITY (CHECK): NORMAL 5 DAYS ON ICE COMMENTS (A) TURNAROUND TIME PTS FILE: -202(82%) TIME PTS QUOTE NO. 24 HOURS 48 HOURS 72 HOURS 100/10 OTHER: INTACT #O4 B RECEIVED COMPANY DATE ATTERBERG LIMITS, ASTM D4318 TOC: WALKLEY-BLACK GRAIN SIZE DISTRIBUTION, ASTM D422/4464M HYDRAULIC CONDUCTIVITY, EPA9100, API RP40, D5084 ANALYSIS REQUEST AND PERMEABILITY, API RP40 BULK DENSITY (DRY), API RP40 of ASTM D2937 SPECIFIC GRAVITY, ASTM D854 POROSITY: EFFECTIVE, ASTM D425M B POROSITY: TOTAL, API RP40 RELINQUISHED MOISTURE CONTENT, ASTM D2216 РНОТОСОВ: СОЯЕ РНОТОВЯРНУ COMPANY FLUID PROPERTIES PACKAGE SAPILEARITY PACKAGE TCEO/TNRCC PROPERTIES PACKAGE PORE FLUID SATURATIONS PACKAGE 7560 HYDRAULIC CONDUCTIVITY PACKAGE SOIL PROPERTIES PACKAGE NUMBER OF SAMPLES TIME ZIP CODE DEPTH, FT (503) 605-106, PHONE NUMBER 0.01-10.0 603)603-1001 97225 10,0'-12. 11000 FAX NUMBER 1900 ✓ 2. RECEIVED BY Q MYOSIISOU TIME N Marine M 87 01000000 20 DATE Shaw Envisormental Environmana/ 5 K ESTE Taccona 30-11-10-0-10-N SC-2-15.0-7.5 50-275100 10300 SW NimbyC. PROJECT MANAGER SAMPLE ID NUMBER にもこの人名ときが SAMPLER SIQNATURE RELINQUISHED BY Marien Ma Marien Ma Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Marien Ma Marien Marien Marien Marien Ma Marien Marien Marien Marien Ma M PROJECT NUMBER 0.500 PROJECT NAME FORM EN SITE LOCATION 202 COMPANY

PTS Laboratories, Inc. • 8100 Secura Way • Santa Fe Springs, CA 90670 • Phone (562) 907-3607 • Fax (562) 907-3610

PTS Genl ahs Inc. • 4342 W 12th St • Houston TX 77055 • Phone (713) 680-2991 • Fax (713) 680-0763

38090

Client:

Shaw Environmental

PHYSICAL PROPERTIES DATA - PORE FLUID SATURATIONS

PROJECT NAME: Port of Tacoma
PROJECT NO: 111487 01000000

API RP 40 / API RP 40 API RP 40 API RP 40 **ASTM D2216** METHODS: POROSITY, %Vb (2) PORE FLUID SAMPLE MOISTURE DENSITY SATURATIONS, % Pv (3) SAMPLE CONTENT. BULK, GRAIN. AIR DEPTH. ORIENTATION TOTAL **FILLED** WATER NAPL % weight g/cc g/cc (1) ID. ft. 79.1 8.1 Н 25.8 1.49 2.70 44.8 5.7 SC-1-8-10.5 8.9 45.6 9.8 69.6 8.8 SC-1-10.5-13.0 11.65 Н 24.1 1.46 2.69 20.2 51.5 2.9 2.69 44.3 SC-1-10.5-13.0 12.2 Η 16.0 1.50 64.2 28.1 8.7 Н 67.4 0.82 2.07 60.5 4.7 SC-2-7.5-10.0 2.00 2.70 26.1 4.8 57.0 24.6 Н 10.4 SC-2-10.0-12.5 10.7 40.1 46.3 SC-2-15.0-17.5 16.1 Н 24.2 1.53 2.73 44.0 6.0 4.0 46.0 45.3 27.9 2.70 45.9 SC-2-15.0-17.5 16.6 1.46

⁽¹⁾ Sample Orientation: H = horizontal; V = vertical (2) Total Porosity = no pore fluids in place; all interconnected pore channels; Air Filled = pore channels not occupied by pore fluids (3) Water = 0.9996 g/cc, SC-1 Hydrocarbon = 0.8803 g/cc, SC-2 Hydrocarbon = 0.9498 g/cc; Vb = Bulk Volume, cc; Pv = Pore Volume, cc; ND = Not Detected

38090

Client:

Shaw Environmental

PHYSICAL PROPERTIES DATA - AIR/WATER CAPILLARY PRESSURE

PROJECT NAME: Port of Tacoma
PROJECT NO: 111487 01000000

		METHODS:	API RP 40 / ASTM D2216	API	RP 40	API F	RP 40	API	RP 40
		SAMPLE	MOISTURE	DEN	ISITY	POROSIT	Y, %Vb (2)	PORE	FLUID
SAMPLE	DEPTH,	ORIENTATION	CONTENT,	BULK,	GRAIN,		AIR	SATURATIO	NS, % Pv (3)
ID.	ft.	(1)	% weight	g/cc	g/cc	TOTAL	FILLED	WATER	NAPL
SC-1-10.5'-13.0'	11.8	Н	24.8	1.46	2.67	45.4	8.5	68.4	13.0
SC-2-15.0'-17.5'	15.95	Н	20.9	1.66	2.79	40.6	5.1	60.2	27.3
SC-2-15.0'-17.5'	16.75	Н	26.7	1.47	2.68	45.2	4.6	38.5	51.2

PHYSICAL PROPERTIES DATA - AIR/WATER CAPILLARY PRESSURE

PROJECT NAME: Port of Tacoma
PROJECT NO: 111487 01000000

ft.

(1)

ID.

API RP 40 / METHODS: **ASTM D2216** API RP 40 API RP 40 API RP 40 MOISTURE DENSITY POROSITY, %Vb (2) PORE FLUID SAMPLE SATURATIONS, % Pv (3) SAMPLE DEPTH, ORIENTATION CONTENT, BULK, GRAIN, AIR

g/cc

TOTAL

FILLED

WATER

NAPL

(1) Sample Orientation: H = horizontal; V = vertical (2) Total Porosity = no pore fluids in place; all interconnected pore channels; Air Filled = pore channels
not occupied by pore fluids (3) Water = 0.9996 g/cc, SC-1 Hydrocarbon = 0.8803 g/cc, SC-2 Hydrocarbon = 0.9498 g/cc; Vb = Bulk Volume, cc; Pv = Pore
Volume, cc: ND = Not Detected

g/cc

% weight

Client:

Shaw Environmental

PERMEABILITY DATA

PROJECT NAME: Port of Tacoma PROJECT NO: 111487 01000000

METHODS: API RP 40; EPA 9100

				25 PSI CONFINING STRESS							
				SPECIFIC (2)	EFFECTIVE (3,4)	HYDRAULIC					
1	SAMPLE	DEPTH,	SAMPLE	PERMEABILITY TO AIR	PERMEABILITY TO WATER,	CONDUCTIVITY (3,4),					
1	ID.	ft.	ORIENTATION (1)	millidarcy	millidarcy	cm/s					
	SC-1-10.5'-13.0'	11.8	Н	5346	2410	2.21E-03					
	SC-2-15.0'-17.5'	15.95	Н	4841	2178	1.98E-03					
	SC-2-15.0'-17.5'	16.75	Н	6139	3858	3.53E-03					

PERMEABILITY DATA

PROJECT NAME: Port of Tacoma
PROJECT NO: 111487 01000000

METHODS: API RP 40; EPA 9100

				25 PSI CONFINING STRESS	
			SPECIFIC (2)	EFFECTIVE (3,4)	HYDRAULIC
SAMPLE	DEPTH,	SAMPLE	PERMEABILITY TO AIR	PERMEABILITY TO WATER,	CONDUCTIVITY (3,4),
ID.	ft.	ORIENTATION (1)	millidarcy	millidarcy	cm/s

(1) Sample Orientation: H = horizontal; V = vertical (2) Specific = No pore fluids in place (3) Native State or Effective = With as-received pore fluids in place (4) Permeability to water and hydraulic conductivity measured at saturated conditions

PTS File No: 38090 Client: Shaw Environmental

FREE PRODUCT MOBILITY: INITIAL AND RESIDUAL SATURATIONS

PROJECT NAME: Port of Tacoma PROJECT NO: 111487 01000000

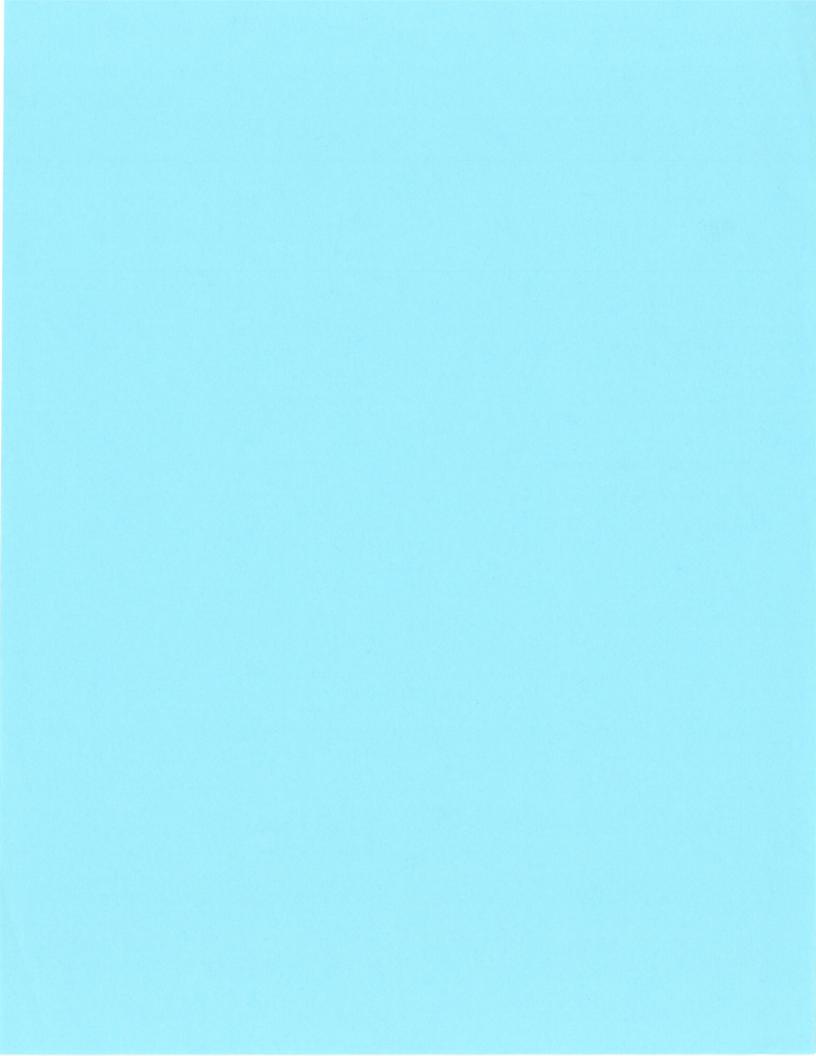
FREE PRODUCT MOBILITY: INITIAL AND RESIDUAL SATURATIONS

Port of Tacoma 111487 01000000 PROJECT NAME: PROJECT NO:

ASTM D425M, DEAN-STARK		After Centrifuge at 1000xG	NAPL (Sor	SATURATIC	,		
	PORE FLUID SATURATIONS, % Pv	After Centrifu	WATER (Srw)	SATURATION			
	PORE FLUID SAT	aturations	Initial Fluid Saturations	PORE FLUID SA	NAPL (Soi)	SATURATION	
		Initial Fluid	WATER (Swi)	SATURATION			
API RP 40		TOTAL	POROSITY,	q//%	, manual		
API RP 40		DENSITY	GRAIN,	g/cc	, , , ,		
API		DEN	BULK,	oo/6	200		
METHODS:		SAMPLE	ORIENTATION	(1)	, ,		
			DEPTH,	Ψ.			
			SAMPLE	ID.	10000 - 10000 - 10000 - 10000 - 10000 - 10000 1		
	٠.						

NAPL (Sor) SATURATION

Water =0.9996 g/cc, SC-1 NAPL = 0.8803 g/cc, SC-2 NAPL = 0.9498 g/cc.



38090

Client:

Shaw Environmental

PHYSICAL PROPERTIES DATA - AIR/WATER CAPILLARY PRESSURE

(ASTM D6836; Centrifugal Method: air displacing water)

PROJECT NO:

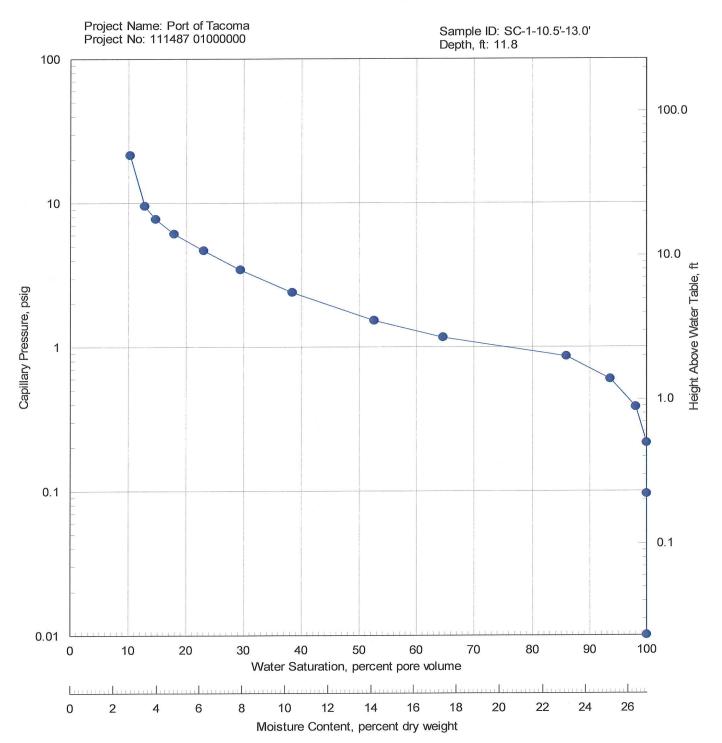
PROJECT NAME: Port of Tacoma 111487 01000000

			Sample ID	
Conillon	Capillary Pressure Height Above		SC-1 (10.5'-1	3.0') at 11.8 ft.
Capillary Fressure		Water Table,	Saturation,	Moisture,
psi	cm water	ft	% pore volume	% dry weight
0.000	0.00	0.000	100.0	26.9
0.095	6.68	0.220	100.0	26.9
0.214	15.0	0.495	100.0	26.9
0.380	26.7	0.880	98.1	26.4
0.594	41.8	1.37	93.6	25.2
0.855	60.1	1.98	85.9	23.1
1.16	81.8	2.69	64.7	17.4
1.52	107	3.52	52.6	14.2
2.38	167	5.50	38.5	10.4
3.42	241	7.92	29.5	7.9
4.66	327	10.8	23.1	6.2
6.08	428	14.1	17.9	4.8
7.70	541	17.8	14.7	4.0
9.50	668	22.0	12.8	3.5
21.4	1503	49.5	10.3	2.8



CAPILLARY PRESSURE

Centrifugal Method
Air Displacing Water System - ASTM D6836



38090

Client:

Shaw Environmental

TABLE 1

SOIL MOISTURE RETENTION CURVE FITTING FOR QUANTIFYING THE HYDRAULIC FUNCTIONS OF UNSATURATED SOILS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-1 (10.5'-13.0') at 11.8 ft.

van Genuchten Parameters

vali oonaonton i araniotoro		
 alpha (1/cm)	1.160E-02	
n	2.602	
residual water	0.095	
total water	1.000	
m	0.616	
Water Perm (mD)	2410	
Air Perm (mD)	5346	
R^2	0.99491	

Notes: alpha, n, and residual saturation are capillary parameters defined by the following equation (van Genuchten, 1980), with m = 1-1/n, S = water saturation, and h = capillary head (cm):

$$S_r + [(S_s - S_r)/(1 + (\alpha h)^n)^m]$$

Laboratory Measured Data

Capillary	SC-1 (10.5'-13.0') at 11.8 ft.	Predicted Moisture	
Head (cm)	Volumetric Moisture	by Curve Fit	RPD (%)
0.00	1.000	1.000	0.00%
6.68	1.000	0.999	-0.07%
15.03	1.000	0.994	-0.59%
26.72	0.981	0.975	-0.64%
41.76	0.936	0.925	-1.20%
60.13	0.859	0.833	-2.98%
81.84	0.647	0.710	9.64%
106.89	0.526	0.580	10.43%
167.02	0.385	0.379	-1.59%
240.51	0.295	0.263	-10.85%
327.36	0.231	0.200	-13.43%
427.57	0.179	0.164	-8.66%
541.15	0.147	0.142	-3.36%
668.08	0.128	0.129	0.58%
1503.19	0.103	0.104	1.68%

PTS File No: Client:

38090

Shaw Environmental

TABLE 2
FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-1 (10.5'-13.0') at 11.8 ft.

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

Capillary	L RELATIONSHIPS BASED ((van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
1 Toda (OIII)	Total Total		
0.001	1.000	2.41E+03	2.46E-19
0.02	1.000	2.41E+03	1.78E-13
0.04	1.000	2.41E+03	4.05E-12
0.05	1.000	2.41E+03	1.11E-11
0.08	1.000	2.41E+03	9.18E-11
0.09	1.000	2.41E+03	1.56E-10
1	1.000	2.41E+03	8.02E-06
1.2	1.000	2.40E+03	1.82E-05
1.3	1.000	2.40E+03	2.61E-05
1.5	1.000	2.40E+03	4.98E-05
2	1.000	2.40E+03	1.82E-04
3	1.000	2.39E+03	1.13E-03
5	1.000	2.36E+03	1.13E-02
7	0.999	2.32E+03	5.13E-02
9	0.998	2.28E+03	1.59E-01
11	0.997	2.23E+03	3.91E-01
15	0.994	2.12E+03	1.56E+00
20	0.988	1.96E+03	5.61E+00
25	0.978	1.78E+03	1.49E+01
30	0.966	1.60E+03	3.26E+01
35	0.950	1.41E+03	6.23E+01
40	0.932	1.23E+03	1.07E+02
45	0.911	1.06E+03	1.70E+02
50	0.887	9.07E+02	2.53E+02
60	0.834	6.42E+02	4.78E+02
70	0.777	4.42E+02	7.74E+02
75	0.749	3.65E+02	9.41E+02
80	0.720	3.00E+02	1.12E+03
85	0.692	2.46E+02	1.30E+03
90	0.665	2.02E+02	1.48E+03
95	0.639	1.66E+02	1.66E+03
100	0.613	1.36E+02	1.85E+03
120	0.524	6.32E+01	2.51E+03
140	0.452	3.07E+01	3.06E+03
160	0.395	1.57E+01	3.49E+03
180	0.351	8.43E+00	3.82E+03
200	0.315	4.76E+00	4.08E+03
220	0.287	2.80E+00	4.28E+03
240	0.263	1.72E+00	4.44E+03
260	0.244	1.09E+00	4.57E+03

38090

Shaw Environmental

Client:

TABLE 2

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-1 (10.5'-13.0') at 11.8 ft.

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

FUNCTIONAL	_ RELATIONSHIPS BASED (ON DERIVED CAPILLARY I	
Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
280	0.228	7.09E-01	4.67E+03
300	0.215	4.75E-01	4.75E+03
320	0.204	3.26E-01	4.82E+03
340	0.194	2.29E-01	4.88E+03
360	0.185	1.63E-01	4.92E+03
380	0.178	1.19E-01	4.97E+03
400	0.172	8.78E-02	5.00E+03
420	0.166	6.57E-02	5.03E+03
440	0.161	4.99E-02	5.06E+03
460	0.156	3.83E-02	5.08E+03
480	0.152	2.98E-02	5.10E+03
500	0.149	2.33E-02	5.12E+03
550	0.141	1.32E-02	5.16E+03
600	0.135	7.87E-03	5.18E+03
650	0.130	4.88E-03	5.21E+03
700	0.127	3.13E-03	5.22E+03
750	0.123	2.07E-03	5.24E+03
800	0.120	1.41E-03	5.25E+03
850	0.118	9.79E-04	5.26E+03
900	0.116	6.95E-04	5.27E+03
950	0.114	5.03E-04	5.28E+03
1000	0.113	3.70E-04	5.28E+03
1050	0.111	2.76E-04	5.29E+03
1100	0.110	2.09E-04	5.29E+03
1150	0.109	1.60E-04	5.30E+03
1200	0.108	1.24E-04	5.30E+03
1250	0.107	9.69E-05	5.30E+03
1300	0.107	7.66E-05	5.31E+03
1350	0.106	6.11E-05	5.31E+03
1400	0.105	4.91E-05	5.31E+03
1450	0.105	3.98E-05	5.31E+03
1500	0.104	3.25E-05	5.31E+03
1550	0.104	2.67E-05	5.32E+03
1600	0.103	2.20E-05	5.32E+03
1650	0.103	1.83E-05	5.32E+03
1700	0.103	1.53E-05	5.32E+03
1750	0.102	1.29E-05	5.32E+03
1800	0.102	1.09E-05	5.32E+03
1850	0.102	9.22E-06	5.32E+03
1900	0.101	7.85E-06	5.33E+03

38090

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Client:

Shaw Environmental

PTS Laboratories

TABLE 2 FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

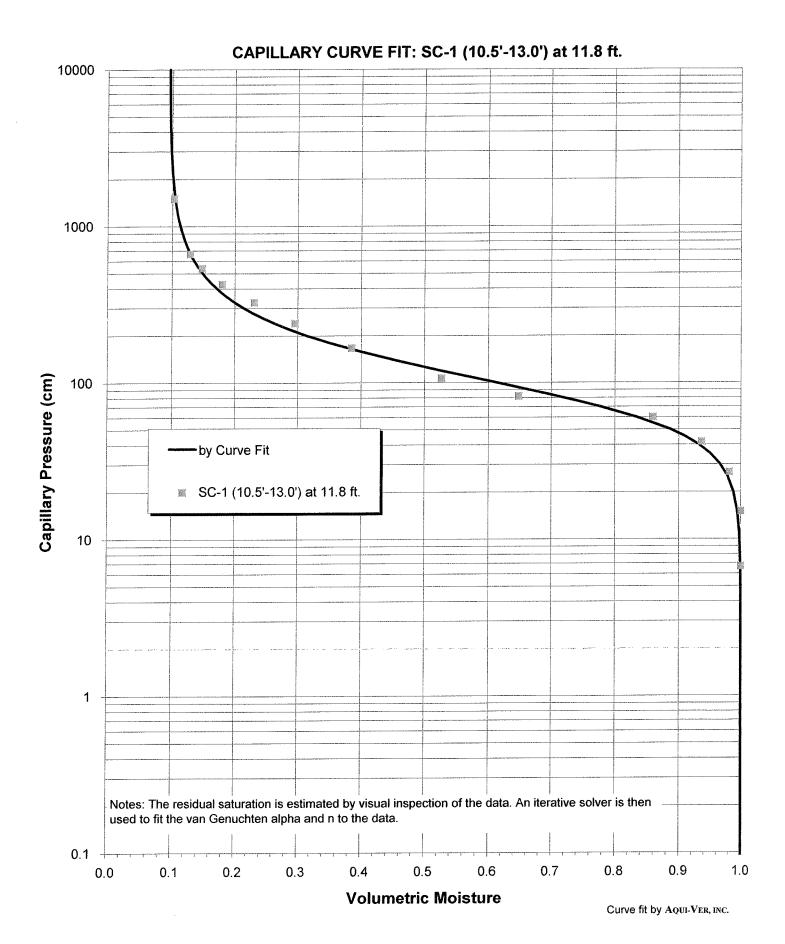
SAMPLE ID:

SC-1 (10.5'-13.0') at 11.8 ft.

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
· · · · · · · · · · · · · · · · · · ·			
1950	0.101	6.72E-06	5.33E+03
2000	0.101	5.77E-06	5.33E+03
2200	0.100	3.26E-06	5.33E+03
2400	0.099	1.93E-06	5.33E+03
2600	0.099	1.19E-06	5.33E+03
2800	0.098	7.66E-07	5.34E+03
3000	0.098	5.06E-07	5.34E+03
3200	0.098	3.43E-07	5.34E+03
3400	0.098	2.39E-07	5.34E+03
3600	0.097	1.69E-07	5.34E+03
3800	0.097	1.22E-07	5.34E+03
4000	0.097	9.00E-08	5.34E+03
4200	0.097	6.71E-08	5.34E+03
4400	0.097	5.08E-08	5.34E+03
4600	0.097	3.89E-08	5.34E+03
4800	0.096	3.01E-08	5.34E+03
5000	0.096	2.36E-08	5.34E+03
5200	0.096	1.86E-08	5.34E+03
5400	0.096	1.48E-08	5.34E+03
5600	0.096	1.19E-08	5.34E+03
5800	0.096	9.66E-09	5.34E+03
6000	0.096	7.88E-09	5.34E+03
6500	0.096	4.88E-09	5.34E+03
7000	0.096	3.12E-09	5.34E+03
7500	0.096	2.06E-09	5.34E+03
8000	0.096	1.40E-09	5.34E+03
8500	0.096	9.74E-10	5.34E+03
9000	0.096	6.91E-10	5.34E+03
9500	0.095	4.99E-10	5.34E+03
10000	0.095	3.67E-10	5.35E+03

Client: Shaw Environmental



Client: Shaw Environmental

PTS File No: 38090

1.0E+04 1.0E+03 1.0E+02 CAPILLARY CURVE FIT: SC-1 (10.5'-13.0') at 11.8 ft. Volumetric Moisture or Effective Permeability 1.0E+01 1.0E+00 1.0E-01 1.0E-02 SC-1 (10.5'-13.0') at 11.8 ft. Effective Water Perm. Effective Air Perm. 1.0E-03 -by Curve Fit 1.0E-04 1.0E-05 0.1 10 10000 1000 100 Capillary Pressure (cm)

Curve fit by AQUI-VER, INC.

Client:

38090 Shaw Environmental

PHYSICAL PROPERTIES DATA - AIR/WATER CAPILLARY PRESSURE

(ASTM D6836; Centrifugal Method: air displacing water)

PROJECT NAME: PROJECT NO:

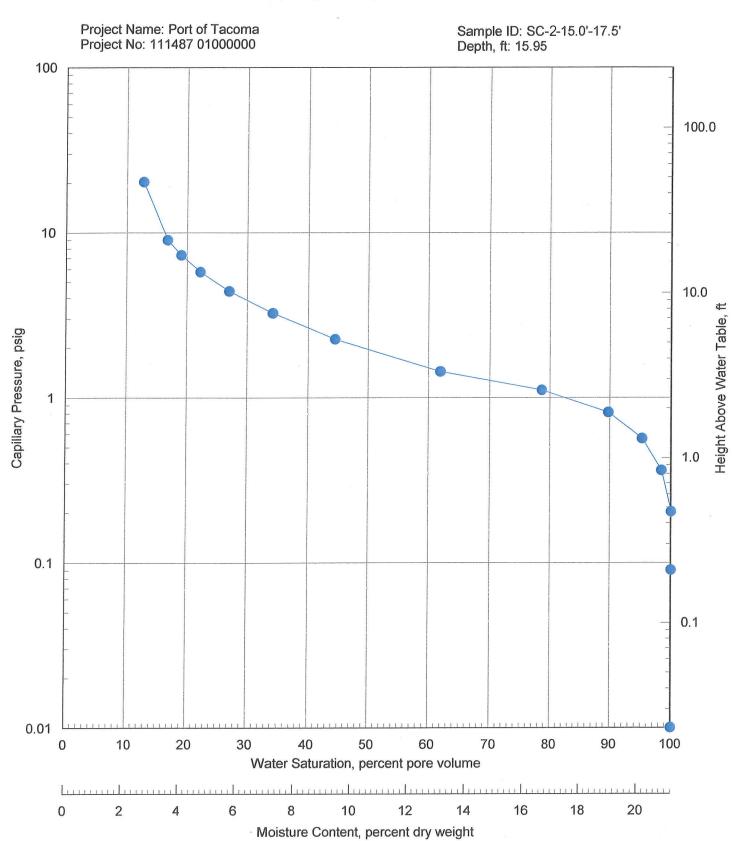
Port of Tacoma 111487 01000000

			Samp	ole ID
Capillary Pressure		Height Above	SC-2 (15.0'-17	.5') at 15.95 ft.
		Water Table,	Saturation,	Moisture,
psi	cm water	ft	% pore volume	% dry weight
<u> </u>				
0.000	0.00	0.000	100.0	21.2
0.090	6.35	0.209	100.0	21.2
0.203	14.3	0.470	100.0	21.2
0.361	25.4	0.836	98.4	20.9
0.564	39.7	1.31	95.2	20.2
0.812	57.1	1.88	89.7	19.1
1.11	77.7	2.56	78.6	16.7
1.44	102	3.34	61.9	13.2
2.26	159	5.22	44.4	9.4
3.25	228	7.52	34.1	7.3
4.42	311	10.2	27.0	5.7
5.78	406	13.4	22.2	4.7
7.31	514	16.9	19.0	4.1
9.03	635	20.9	16.7	3.5
20.3	1428	47.0	12.7	2.7



CAPILLARY PRESSURE Centrifugal Method

Air Displacing Water System - ASTM D6836



38090

Client:

Shaw Environmental

TABLE 1

SOIL MOISTURE RETENTION CURVE FITTING FOR QUANTIFYING THE HYDRAULIC FUNCTIONS OF UNSATURATED SOILS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 15.95 ft.

van Genuchten Parameters

 alpha (1/cm)	1.046E-02	
n	2.516	
residual water	0.110	
total water	1.000	
m	0.603	
Water Perm (mD)	2178	
Air Perm (mD)	4841	
R ²	0.99781	

Notes: alpha, n, and residual saturation are capillary parameters defined by the following equation (van Genuchten, 1980), with m = 1-1/n, S = water saturation, and h = capillary head (cm):

$$S_r + [(S_s - S_r)/(1 + (\alpha h)^n)^m]$$

Laboratory Measured Data

SC-2 (45 0'-17 5') at 15 95 ft	Predicted Moisture	
1 ' 1		RPD (%)
1.000	1.000	0.00%
1.000	0.999	-0.06%
1.000	0.996	-0.45%
0.984	0.981	-0.27%
0.952	0.946	-0.66%
0.897	0.879	-1.96%
0.786	0.782	-0.49%
0.619	0.669	8.06%
0.444	0.466	4.82%
0.341	0.333	-2.47%
0.270	0.254	-5.72%
0.222	0.208	-6.51%
0.190	0.179	-6.09%
0.167	0.160	-3.87%
0.127	0.125	-1.76%
	1.000 0.984 0.952 0.897 0.786 0.619 0.444 0.341 0.270 0.222 0.190 0.167	Volumetric Moisture by Curve Fit 1.000 1.000 1.000 0.999 1.000 0.996 0.984 0.981 0.952 0.946 0.897 0.879 0.786 0.782 0.619 0.669 0.444 0.466 0.341 0.333 0.270 0.254 0.222 0.208 0.190 0.179 0.167 0.160

38090

Shaw Environmental

Client:

TABLE 2

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 15.95 ft.

Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
Tread (citi)	volumente Mostare	Lileotive vvacer i emi.	Lincoave 7 til 1 ciril.
0.001	1.000	2.18E+03	1.62E-18
0.02	1.000	2.18E+03	6.16E-13
0.04	1.000	2.18E+03	1.21E-11
0.05	1.000	2.18E+03	3.14E-11
0.08	1.000	2.18E+03	2.36E-10
0.09	1.000	2.18E+03	3.91E-10
1	1.000	2.17E+03	1.20E-05
1.2	1.000	2.17E+03	2.62E-05
1.3	1.000	2.17E+03	3.69E-05
1.5	1.000	2.17E+03	6.83E-05
2	1.000	2.17E+03	2.35E-04
3	1.000	2.16E+03	1.34E-03
5	1.000	2.13E+03	1.19E-02
7	0.999	2.10E+03	5.05E-02
9	0.999	2.06E+03	1.48E-01
11	0.998	2.01E+03	3.50E-01
15	0.995	1.92E+03	1.31E+00
20	0.990	1.78E+03	4.43E+00
25	0.982	1.64E+03	1.13E+01
30	0.972	1.49E+03	2.39E+01
35	0.960	1.33E+03	4.46E+01
40	0.945	1.19E+03	7.55E+01
45	0.928	1.04E+03	1.18E+02
50	0.909	9.10E+02	1.75E+02
60	0.866	6.77E+02	3.30E+02
70	0.820	4.92E+02	5.39E+02
75	0.795	4.17E+02	6.60E+02
80	0.771	3.52E+02	7.89E+02
85	0.747	2.97E+02	9.26E+02
90	0.722	2.50E+02	1.07E+03
95	0.699	2.10E+02	1.21E+03
100	0.676	1.77E+02	1.35E+03
120	0.591	8.95E+01	1.91E+03
140	0.520	4.66E+01	2.40E+03
160	0.462	2.52E+01	2.81E+03
180	0.415	1.42E+01	3.14E+03
200	0.376	8.34E+00	3.40E+03
220	0.345	5.08E+00	3.61E+03
240	0.318	3.20E+00	3.78E+03
260	0.296	2.08E+00	3.92E+03

PTS File No: 38090 PTS Laboratories

Client: Shaw Environmental

TABLE 2 FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME: Port of Tacoma
PROJECT NO: 111487 01000000

SAMPLE ID: SC-2 (15.0'-17.5') at 15.95 ft.

Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
Tiead (CIII)	Volumetric Moistare	Liteative vvaler i eiii.	Liteotive 7th 1 cm.
280	0.278	1.39E+00	4.03E+03
300	0.262	9.48E-01	4.13E+03
320	0.249	6.62E-01	4.21E+03
340	0.237	4.72E-01	4.27E+03
360	0.227	3.42E-01	4.33E+03
380	0.218	2.53E-01	4.37E+03
400	0.210	1.89E-01	4.42E+03
420	0.203	1.43E-01	4.45E+03
440	0.197	1.10E-01	4.48E+03
460	0.191	8.54E-02	4.51E+03
480	0.186	6.70E-02	4.53E+03
500	0.182	5.31E-02	4.56E+03
550	0.172	3.07E-02	4.60E+03
600	0.165	1.87E-02	4.64E+03
650	0.158	1.18E-02	4.66E+03
700	0.153	7.69E-03	4.68E+03
750	0.149	5.17E-03	4.70E+03
800	0.145	3.56E-03	4.72E+03
850	0.142	2.51E-03	4.73E+03
900	0.140	1.81E-03	4.74E+03
950	0.137	1.32E-03	4.75E+03
1000	0.135	9.82E-04	4.76E+03
1050	0.133	7.41E-04	4.76E+03
1100	0.132	5.66E-04	4.77E+03
1150	0.130	4.38E-04	4.77E+03
1200	0.129	3.42E-04	4.78E+03
1250	0.128	2.70E-04	4.78E+03
1300	0.127	2.16E-04	4.79E+03
1350	0.126	1.73E-04	4.79E+03
1400	0.125	1.40E-04	4.79E+03
1450	0.124	1.15E-04	4.80E+03
1500	0.124	9.42E-05	4.80E+03
1550	0.123	7.79E-05	4.80E+03
1600	0.122	6.48E-05	4.80E+03
1650	0.122	5.43E-05	4.80E+03
1700	0.121	4.57E-05	4.81E+03
1750	0.121	3.86E-05	4.81E+03
1800	0.120	3.28E-05	4.81E+03
1850	0.120	2.80E-05	4.81E+03
1900	0.120	2.40E-05	4.81E+03

38090

Client:

Shaw Environmental

TABLE 2 FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

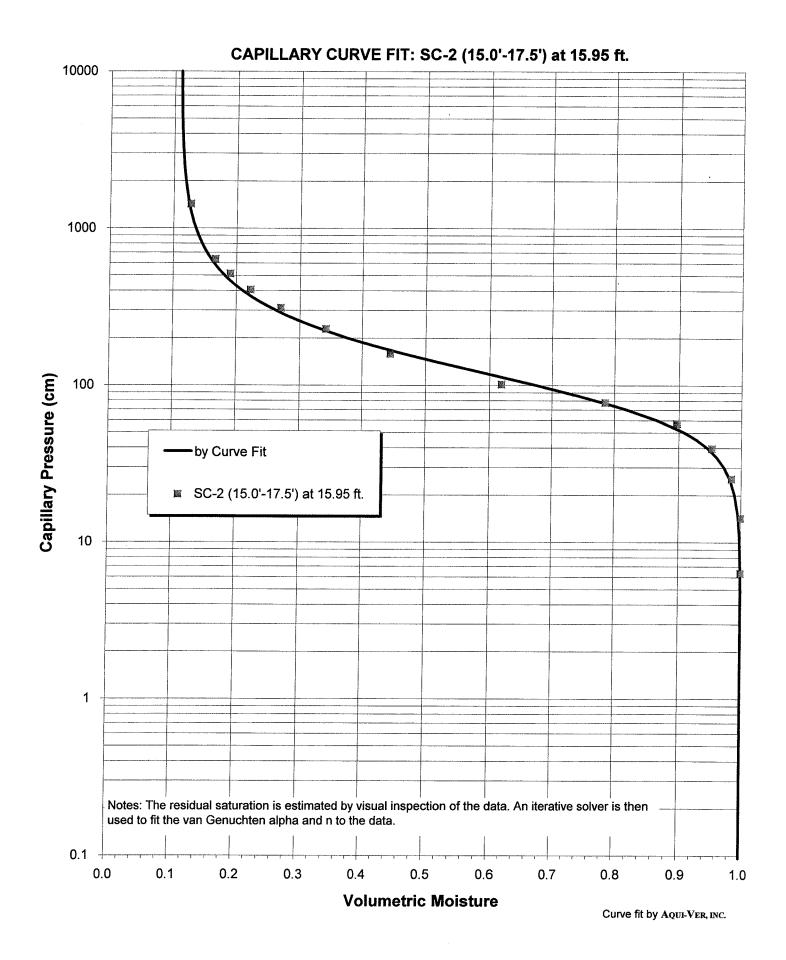
111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 15.95 ft.

Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
	-		
1950	0.119	2.06E-05	4.81E+03
2000	0.119	1.78E-05	4.81E+03
2200	0.118	1.03E-05	4.82E+03
2400	0.117	6.20E-06	4.82E+03
2600	0.116	3.90E-06	4.82E+03
2800	0.115	2.54E-06	4.83E+03
3000	0.115	1.70E-06	4.83E+03
3200	0.114	1.17E-06	4.83E+03
3400	0.114	8.26E-07	4.83E+03
3600	0.114	5.93E-07	4.83E+03
3800	0.113	4.34E-07	4.83E+03
4000	0.113	3.22E-07	4.83E+03
4200	0.113	2.43E-07	4.83E+03
4400	0.113	1.86E-07	4.83E+03
4600	0.113	1.44E-07	4.83E+03
4800	0.112	1.12E-07	4.83E+03
5000	0.112	8.86E-08	4.84E+03
5200	0.112	7.06E-08	4.84E+03
5400	0.112	5.67E-08	4.84E+03
5600	0.112	4.60E-08	4.84E+03
5800	0.112	3.75E-08	4.84E+03
6000	0.112	3.08E-08	4.84E+03
6500	0.111	1.94E-08	4.84E+03
7000	0.111	1.26E-08	4.84E+03
7500	0.111	8.47E-09	4.84E+03
8000	0.111	5.83E-09	4.84E+03
8500	0.111	4.10E-09	4.84E+03
9000	0.111	2.95E-09	4.84E+03
9500	0.111	2.15E-09	4.84E+03
10000	0.111	1.60E-09	4.84E+03

Client: Shaw Environmental



Client: Shaw Environmental

PTS File No: 38090

1.0E+04 Curve fit by AQUI-VER, INC. 1.0E+03 1.0E+02 CAPILLARY CURVE FIT: SC-2 (15.0'-17.5') at 15.95 ft. Volumetric Moisture or Effective Permeability 1.0E+01 1.0E+00 1.0E-01 1.0E-02 SC-2 (15.0'-17.5') at 15.95 ft. Effective Water Perm. Effective Air Perm. 1.0E-03 -by Curve Fit 1.0E-04 1.0E-05 0.1 10 100 10000 1000 Capillary Pressure (cm)

38090

Client:

Shaw Environmental

PHYSICAL PROPERTIES DATA - AIR/WATER CAPILLARY PRESSURE

(ASTM D6836; Centrifugal Method: air displacing water)

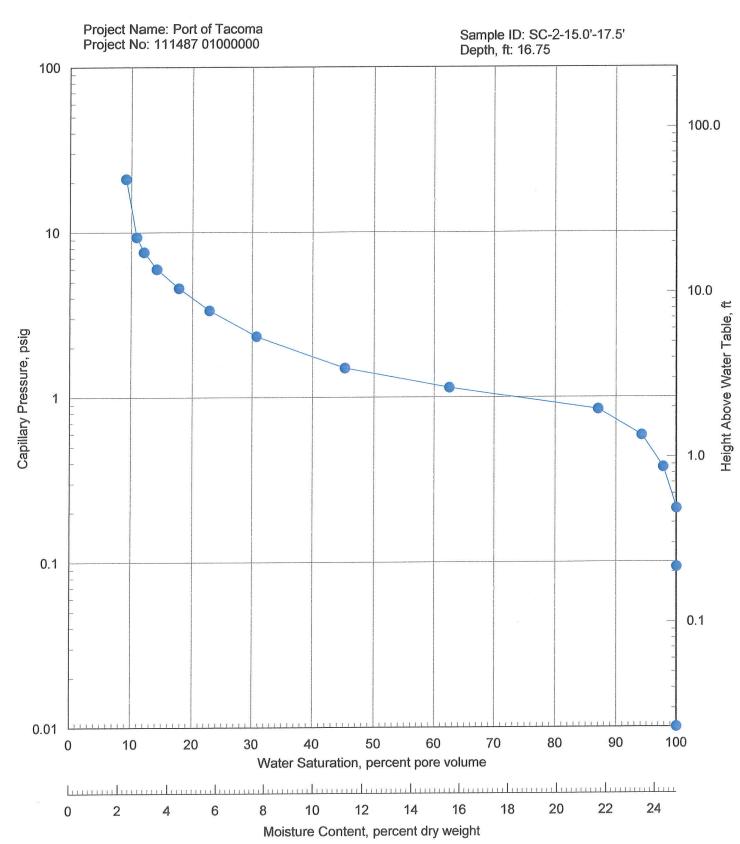
PROJECT NAME: PROJECT NO:

Port of Tacoma 111487 01000000

			Samp	
O - ill - Dresser		Height Above	SC-2 (15.0'-17.5') at 16.75 ft.	
Capillary	/ Pressure	Water Table,	Saturation,	Moisture,
psi	cm water	ft	% pore volume	% dry weight
				010
0.000	0.00	0.000	100.0	24.9
0.093	6.57	0.216	100.0	24.9
0.210	14.8	0.487	100.0	24.9
0.374	26.3	0.865	97.8	24.4
0.584	41.1	1.35	94.2	23.5
0.841	59.1	1.95	87.0	21.7
1.14	80.5	2.65	62.5	15.6
1.50	105	3.46	45.2	11.3
2.34	164	5.41	30.7	7.7
3.36	237	7.79	22.8	5.7
4.58	322	10.6	17.7	4.4
5.98	420	13.8	14.1	3.5
7.57	532	17.5	12.0	3.0
9.34	657	21.6	10.8	2.7
21.0	1478	48.7	9.1	2.3



CAPILLARY PRESSURE Centrifugal Method Air Displacing Water System - ASTM D6836



38090

Shaw Environmental

Client:

TABLE 1

SOIL MOISTURE RETENTION CURVE FITTING FOR QUANTIFYING THE HYDRAULIC FUNCTIONS OF UNSATURATED SOILS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 16.75 ft.

van Genuchten Parameters

vali Gelluciileli Faiailieleis		
 alpha (1/cm)	1.266E-02	
n	3.172	
residual water	0.091	
total water	1.000	
m	0.685	
Water Perm (mD)	3858	
Air Perm (mD)	6139	
R ²	0.99608	

Notes: alpha, n, and residual saturation are capillary parameters defined by the following equation (van Genuchten, 1980), with m = 1-1/n, S = water saturation, and h = capillary head (cm):

$$S_r + [(S_s - S_r)/(1 + (\alpha h)^n)^m]$$

Laboratory Measured Data

Capillary	SC-2 (15.0'-17.5') at 16.75 ft.	Predicted Moisture	
Head (cm)	Volumetric Moisture	by Curve Fit	RPD (%)
0.00	1.000	1.000	0.00%
6.57	1.000	1.000	-0.02%
14.78	1.000	0.997	-0.30%
26.28	0.978	0.982	0.32%
41.06	0.942	0.929	-1.37%
59.13	0.870	0.813	-6.52%
80.48	0.625	0.645	3.26%
105.12	0.452	0.479	5.96%
164.25	0.307	0.265	-13.78%
236.52	0.228	0.173	-23.98%
321.92	0.177	0.134	-24.68%
420.47	0.141	0.115	<i>-</i> 18.67%
532.16	0.120	0.105	-11.98%
656.99	0.108	0.100	<i>-</i> 7.48%
1478.22	0.091	0.093	1.83%

Client:

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Shaw Environmental

TABLE 2
FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 16.75 ft.

FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS					
Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)		
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.		
tenno.					
0.001	1.000	3.86E+03	5.84E-26		
0.02	1.000	3.86E+03	2.41E-18		
0.04	1.000	3.86E+03	1.47E-16		
0.05	1.000	3.86E+03	5.51E-16		
0.08	1.000	3.86E+03	8.93E-15		
0.09	1.000	3.86E+03	1.80E-14		
1	1.000	3.86E+03	2.85E-08		
1.2	1.000	3.86E+03	8.39E-08		
1.3	1.000	3.86E+03	1.35E-07		
1.5	1.000	3.86E+03	3.15E-07		
2	1.000	3.86E+03	1.74E-06		
3	1.000	3.85E+03	1.92E-05		
5	1.000	3.84E+03	3.97E-04		
7	1.000	3.82E+03	2.92E-03		
9	0.999	3.79E+03	1.29E-02		
11	0.999	3.75E+03	4.24E-02		
15	0.997	3.65E+03	2.65E-01		
20	0.992	3.47E+03	1.44E+00		
25	0.984	3.23E+03	5.28E+00		
30	0.972	2.95E+03	1.50E+01		
35	0.956	2.64E+03	3.57E+01		
40	0.934	2.31E+03	7.38E+01		
45	0.908	1.98E+03	1.37E+02		
50	0.878	1.66E+03	2.31E+02		
60	0.807	1.10E+03	5.31E+02		
70	0.728	6.87E+02	9.75E+02		
75	0.688	5.34E+02	1.24E+03		
80	0.649	4.13E+02	1.52E+03		
85	0.611	3.17E+02	1.80E+03		
90	0.575	2.43E+02	2.09E+03		
95	0.541	1.86E+02	2.38E+03		
100	0.509	1.42E+02	2.65E+03		
120	0.403	5.02E+01	3.60E+03		
140	0.328	1.90E+01	4.29E+03		
160	0.274	7.79E+00	4.77E+03		
180	0.236	3.45E+00	5.11E+03		
200	0.208	1.64E+00	5.34E+03		
220	0.187	8.32E-01	5.51E+03		
240	0.171	4.44E-01	5.63E+03		
260	0.158	2.48E-01	5.73E+03		

38090

Shaw Environmental

Client:

TABLE 2 FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 16.75 ft.

Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.
Head (CIII)	Volumetric Moisture	Endate Trata.	
280	0.149	1.44E-01	5.80E+03
300	0.141	8.71E-02	5.85E+03
320	0.134	5.42E-02	5.90E+03
340	0.129	3.47E-02	5.93E+03
360	0.125	2.28E-02	5.96E+03
380	0.121	1.53E-02	5.98E+03
400	0.118	1.05E-02	6.00E+03
420	0.115	7.29E-03	6.02E+03
440	0.113	5.17E-03	6.03E+03
460	0.111	3.72E-03	6.04E+03
480	0.109	2.71E-03	6.05E+03
500	0.108	2.01E-03	6.06E+03
550	0.104	9.89E-04	6.08E+03
600	0.102	5.19E-04	6.09E+03
650	0.100	2.87E-04	6.10E+03
700	0.099	1.65E-04	6.10E+03
750	0.098	9.90E-05	6.11E+03
800	0.097	6.13E-05	6.11E+03
850	0.096	3.91E-05	6.12E+03
900	0.096	2.56E-05	6.12E+03
950	0.095	1.71E-05	6.12E+03
1000	0.095	1.17E-05	6.12E+03
1050	0.094	8.14E-06	6.13E+03
1100	0.094	5.76E-06	6.13E+03
1150	0.094	4.14E-06	6.13E+03
1200	0.093	3.02E-06	6.13E+03
1250	0.093	2.23E-06	6.13E+03
1300	0.093	1.67E-06	6.13E+03
1350	0.093	1.26E-06	6.13E+03
1400	0.093	9.61E-07	6.13E+03
1450	0.093	7.40E-07	6.13E+03
1500	0.093	5.76E-07	6.13E+03
1550	0.092	4.51E-07	6.13E+03
1600	0.092	3.56E-07	6.13E+03
1650	0.092	2.84E-07	6.13E+03
1700	0.092	2.27E-07	6.13E+03
1750	0.092	1.83E-07	6.13E+03
1800	0.092	1.49E-07	6.13E+03
1850	0.092	1.21E-07	6.14E+03
1900	0.092	9.94E-08	6.14E+03

38090

Shaw Environmental

Client:

TABLE 2 FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS

PROJECT NAME:

Port of Tacoma

PROJECT NO:

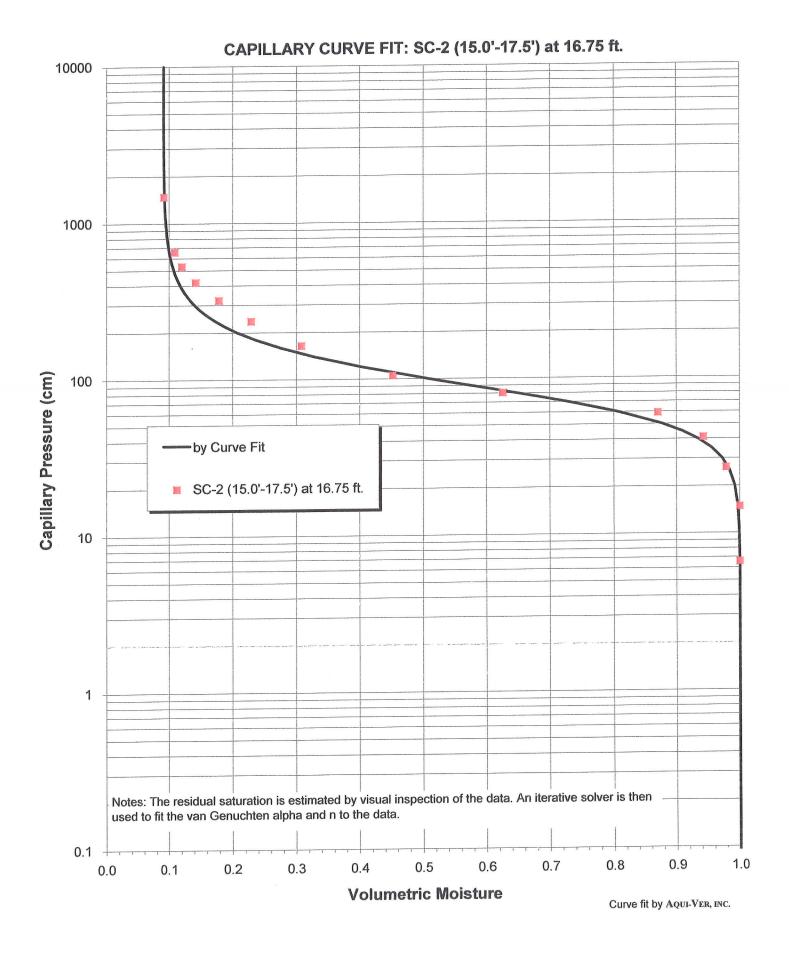
111487 01000000

SAMPLE ID:

SC-2 (15.0'-17.5') at 16.75 ft.

FUNCTIONA	FUNCTIONAL RELATIONSHIPS BASED ON DERIVED CAPILLARY PARAMETERS				
Capillary	(van Genuchten, 1980)	(Mualem, 1976)	(Mualem, 1976)		
Head (cm)	Volumetric Moisture	Effective Water Perm.	Effective Air Perm.		
<u> </u>					
1950	0.092	8.20E-08	6.14E+03		
2000	0.092	6.79E-08	6.14E+03		
2200	0.092	3.35E-08	6.14E+03		
2400	0.092	1.75E-08	6.14E+03		
2600	0.091	9.67E-09	6.14E+03		
2800	0.091	5.58E-09	6.14E+03		
3000	0.091	3.34E-09	6.14E+03		
3200	0.091	2.07E-09	6.14E+03		
3400	0.091	1.32E-09	6.14E+03		
3600	0.091	8.62E-10	6.14E+03		
3800	0.091	5.77E-10	6.14E+03		
4000	0.091	3.94E-10	6.14E+03		
4200	0.091	2.74E-10	6.14E+03		
4400	0.091	1.94E-10	6.14E+03		
4600	0.091	1.40E-10	6.14E+03		
4800	0.091	1.02E-10	6.14E+03		
5000	0.091	7.51E-11	6.14E+03		
5200	0.091	5.61E-11	6.14E+03		
5400	0.091	4.24E-11	6.14E+03		
5600	0.091	3.24E-11	6.14E+03		
5800	0.091	2.49E-11	6.14E+03		
6000	0.091	1.94E-11	6.14E+03		
6500	0.091	1.07E-11	6.14E+03		
7000	0.091	6.17E-12	6.14E+03		
7500	0.091	3.69E-12	6.14E+03		
8000	0.091	2.29E-12	6.14E+03		
8500	0.091	1.46E-12	6.14E+03		
9000	0.091	9.54E-13	6.14E+03		
9500	0.091	6.38E-13	6.14E+03		
10000	0.091	4.36E-13	6.14E+03		

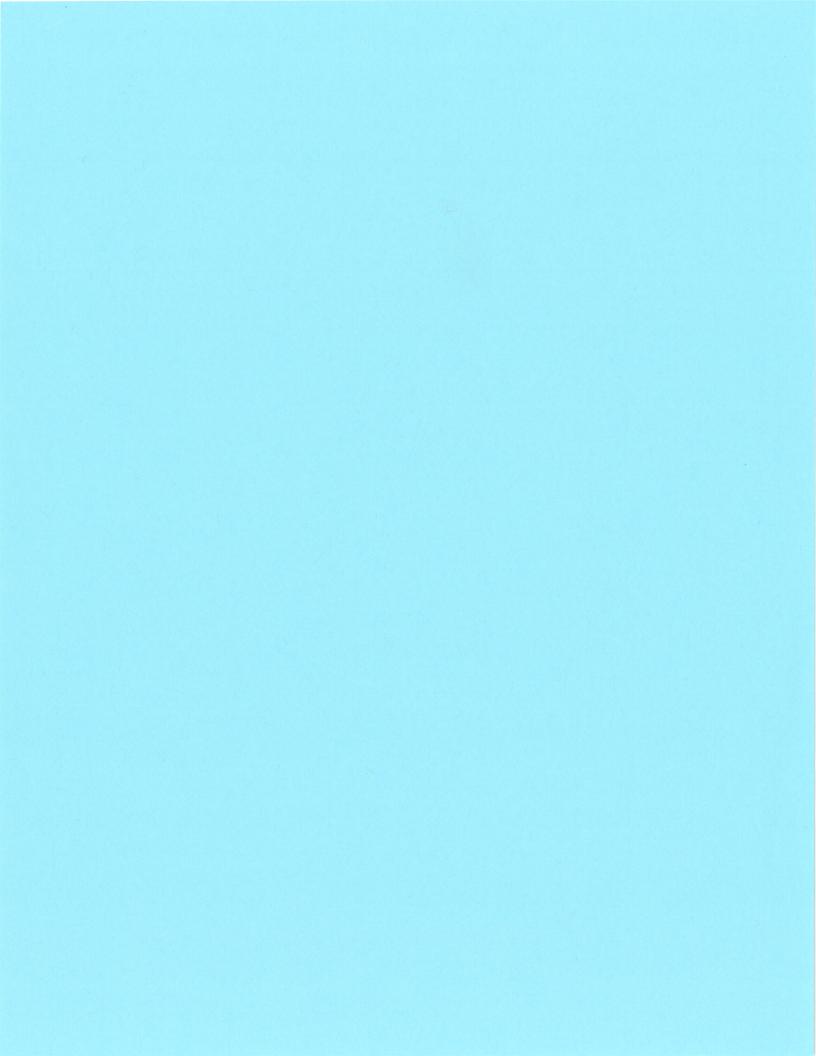
Client: Shaw Environmental



1.0E+04 1.0E+03 1.0E+02 CAPILLARY CURVE FIT: SC-2 (15.0'-17.5') at 16.75 ft. Volumetric Moisture or Effective Permeability 1.0E+01 1.0E+00 1.0E-01 1.0E-02 SC-2 (15.0'-17.5') at 16.75 ft. Effective Water Perm. Effective Air Perm. 1.0E-03 -by Curve Fit 1.0E-04 1.0E-05 9 10000 100 1000 Capillary Pressure (cm)

PTS File No: 38090 Client: Shaw Environmental

Curve fit by AQUI-VER, INC.



38090 Shaw Environmental PTS File No:

PARTICLE SIZE SUMMARY

(METHODOLOGY: ASTM D422/D4464M)

PROJECT NAME: PROJECT NO:

111487 01000000 Port of Tacoma

11.32 4.58 Clay 5.19 3.66 9.21 4.92 9.61 S<u>i</u>t ⊗ 1.25 1.15 Clay 1.39 1.97 0.81 \odot (5)2.85 3.43 3.80 7.24 Silt 3.67 Particle Size Distribution, wt. percent (5)(5)39.43 34.30 Fine 54.75 75.59 53.86 19.71 49.51 Sand Size Medium 40.06 15.20 41.22 40.33 13.11 62.03 45.91 Coarse 27.92 0.00 0.00 0.00 3.00 0.00 0.00 Gravel 33.14 0.00 0.00 0.00 2.43 0.00 0.00 **Grain Size** Median 0.335 0.215 0.348 0.410 0.389 0.504 1.061 шш Mean Grain Size Description (1) Medium sand Medium sand Coarse sand Fine sand Fine sand Fine sand Fine sand Depth, ft. 11.65 12.2 16.6 10.7 16.1 8.9 8.7 SC-1-10.5'-13.0' SC-1-10.5'-13.0' SC-2-15.0'-17.5' SC-2-15.0'-17.5' SC-2-10.0-12.5 SC-2-7.5-10.0 SC-1-8'-10.5' Sample ID

Based on Mean from Trask
 Mechanical sieve does not differentiate silt/clay fractions.

Particle Size Analysis - ASTM D4464M

Client: Project:

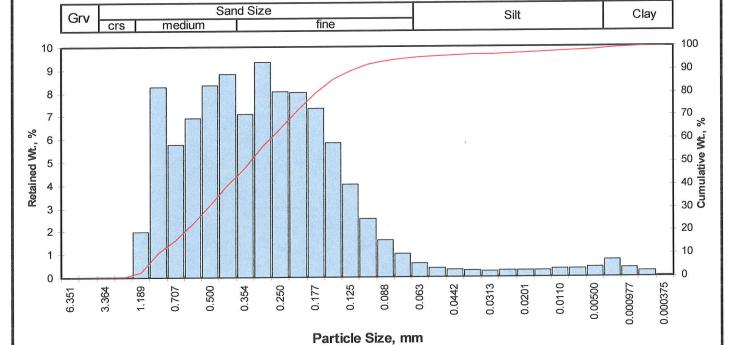
Project No:

Shaw Environmental Port of Tacoma

111487 01000000

PTS File No: Sample ID: 38090 SC-1-8'-10.5'

Sample ID: SC-1-8'-10.5' **Depth, ft:** 8.9



				Sample	Increment	Cumulative
Ope	ning	Phi of	U.S.	Weight,	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
				0.00	0.00	0.00
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	6	0.00	0.00	0.00
0.1324	3.364	-1.75	10	0.00	0.00	0.00
0.0787	2.000	-1.00		1.99	1.99	1.99
0.0468	1.189	-0.25	16		8.26	10.25
0.0331	0.841	0.25	20	8.26 5.76	5.76	16.01
0.0278	0.707	0.50	25		6.90	22.91
0.0234	0.595	0.75	30	6.90	8.34	31.25
0.0197	0.500	1.00	35	8.34	8.34	40.06
0.0166	0.420	1.25	40 45	8.81 7.07	7.07	40.06
0.0139	0.354	1.50 1.75	50	9.34	9.34	56.48
0.0117	0.297		60	8.04	8.04	64.52
0.0098	0.250	2.00			8.02	72.54
0.0083	0.210	2.25	70	8.02	8.02 7.31	72.5 4 79.85
0.0070	0.177	2.50	80	7.31	0.000.0	
0.0059	0.149	2.75	100	5.82	5.82	85.67
0.0049	0.125	3.00	120	4.03	4.03	89.70 92.23
0.0041	0.105	3.25	140	2.53	2.53	
0.0035	0.088	3.50	170	1.59	1.59	93.82
0.0029	0.074	3.75	200	0.99	0.99	94.81
0.0025	0.063	4.00	230	0.59	0.59	95.40
0.0021	0.053	4.25	270	0.39	0.39	95.79
0.00174	0.0442	4.50	325	0.31 0.28	0.31 0.28	96.10 96.38
0.00146	0.0372	4.75	400			
0.00123	0.0313	5.00	450	0.25 0.29	0.25 0.29	96.63 96.92
0.000986	0.0250	5.32	500 635	0.29	0.29	97.20
0.000790	0.0201	5.64	035			97.48
0.000615	0.0156	6.00		0.28 0.36	0.28 0.36	97.46
0.000435	0.0110	6.50			0.35	98.19
0.000308	0.00781	7.00		0.35		E 200 N 5
0.000197	0.00500	7.65		0.42	0.42	98.61 99.33
0.000077	0.00195	9.00		0.72	0.72	99.33
0.000038	0.000977	10.00		0.38		99.71
0.000019	0.000488	11.00		0.26 0.03	0.26 0.03	100.00
0.000015	0.000375	11.38				100.00
TOTALS				100.00	100.00	100.00

Cumulative Weight Percent greater than				
Weight	Phi	Parti	cle Size	
percent	Value	Inches	Millimeters	
5	-0.07	0.0413	1.048	
10	0.23	0.0335	0.850	
16	0.50	0.0278	0.707	
25	0.81	0.0224	0.569	
40	1.25	0.0166	0.421	
50	1.58	0.0132	0.335	
60	1.86	0.0108	0.276	
75	2.33	0.0078	0.198	
84	2.68	0.0062	0.156	
90	3.03	0.0048	0.122	
95	3.83	0.0028	0.070	

Measure	Trask	Inman	Folk-Ward
Median, phi	1.58	1.58	1.58
Median, in.	0.0132	0.0132	0.0132
Median, mm	0.335	0.335	0.335
Mean, phi	1.38	1.59	1.58
Mean, in.	0.0151	0.0131	0.0131
Mean, mm	0.384	0.332	0.333
			4.405
Sorting	1.694	1.089	1.135
Skewness	1.002	0.011	0.084
Kurtosis	0.255	0.789	1.050
Grain Size D	escription		Fine sand

Description	Retained on Sieve #	Weight Percent
Gravel	4	0.00
Coarse Sand	10	0.00
Medium Sand	40	40.06
Fine Sand	200	54.75
Silt	>0.005 mm	3.80
Clay	<0.005 mm	1.39
	Total	100

(ASTM-USCS Scale)

(based on Mean from Trask)

Particle Size Analysis - ASTM D4464M

Client: Project:

Project No:

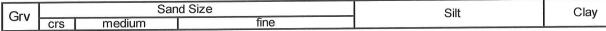
Shaw Environmental

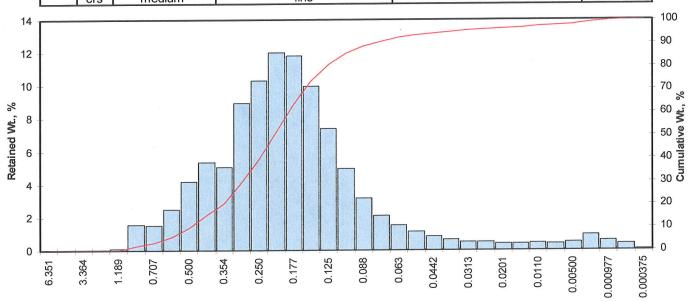
Port of Tacoma 111487 01000000 PTS File No:

Sample ID: Depth, ft:

SC-1-10.5'-13.0'

11.65





Particle Size, mm

				Sample	Increment	
Ope	ening	Phi of	U.S.	Weight,	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	4	0.00	0.00	0.00
0.1324	3.364	-1.75	6	0.00	0.00	0.00
0.0787	2.000	-1.00	10	0.00	0.00	0.00
0.0468	1.189	-0.25	16	0.11	0.11	0.11
0.0331	0.841	0.25	20	1.57	1.57	1.68
0.0278	0.707	0.50	25	1.50	1.50	3.18
0.0234	0.595	0.75	30	2.50	2.50	5.68
0.0197	0.500	1.00	35	4.20	4.20	9.87
0.0166	0.420	1.25	40	5.33	5.33	15.20
0.0139	0.354	1.50	45	5.08	5.08	20.28
0.0117	0.297	1.75	50	8.93	8.93	29.20
0.0098	0.250	2.00	60	10.30	10.29	39.50
0.0083	0.210	2.25	70	12.00	11.99	51.49
0.0070	0.177	2.50	80	11.80	11.79	63.29
0.0059	0.149	2.75	100	9.95	9.94	73.23
0.0049	0.125	3.00	120	7.37	7.37	80.60
0.0041	0.105	3.25	140	4.95	4.95	85.54
0.0035	0.088	3.50	170	3.17	3.17	88.71
0.0029	0.074	3.75	200	2.08	2.08	90.79
0.0025	0.063	4.00	230	1.51	1.51	92.30
0.0021	0.053	4.25	270	1.14	1.14	93.44
0.00174	0.0442	4.50	325	0.83	0.83	94.27
0.00146	0.0372	4.75	400	0.62	0.62	94.89
0.00123	0.0313	5.00	450	0.49	0.49	95.38
0.000986	0.0250	5.32	500	0.51	0.51	95.89
0.000790	0.0201	5.64	635	0.41	0.41	96.30
0.000615	0.0156	6.00		0.38	0.38	96.68
0.000435	0.0110	6.50		0.44	0.44	97.12
0.000308	0.00781	7.00		0.41	0.41	97.53
0.000197	0.00500	7.65		0.50	0.50	98.03
0.000077	0.00195	9.00		0.94	0.94	98.97
0.000038	0.000977	10.00		0.58	0.58	99.55
0.000019	0.000488	11.00		0.41	0.41	99.96
0.000015	0.000375	11.38		0.05	0.04	100.00
TOTALS				100.10	100.00	100.00

Cumulative Weight Percent greater than				
Weight	Phi	Particle Size		
percent	Value	Inches	Millimeters	
5	0.68	0.0245	0.623	
10	1.01	0.0196	0.498	
16	1.29	0.0161	0.409	
25	1.63	0.0127	0.323	
40	2.01	0.0098	0.248	
50	2.22	0.0085	0.215	
60	2.43	0.0073	0.186	
75	2.81	0.0056	0.143	
84	3.17	0.0044	0.111	
90	3.65	0.0031	0.079	
95	4.81	0.0014	0.036	

Measure	Trask	Inman	Folk-Ward
Median, phi	2.22	2.22	2.22
Median, in.	0.0085	0.0085	0.0085
Median, mm	0.215	0.215	0.215
Mean, phi	2.10	2.23	2.23
Mean, in.	0.0092	0.0084	0.0084
Mean, mm	0.233	0.213	0.214
Sorting	1.504	0.941	1.096
Skewness	0.998	0.012	0.134
Kurtosis	0.215	1.191	1.435
Contin Cine D	a a a windia n		Eino cand

Grain Size Description	Fine sand
(ASTM-USCS Scale)	(based on Mean from Trask)

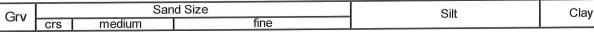
Description	Retained	Weight
	on Sieve #	Percent
Gravel	4	0.00
Coarse Sand	10	0.00
Medium Sand	40	15.20
Fine Sand	200	75.59
Silt	>0.005 mm	7.24
Clay	<0.005 mm	1.97
	Total	100

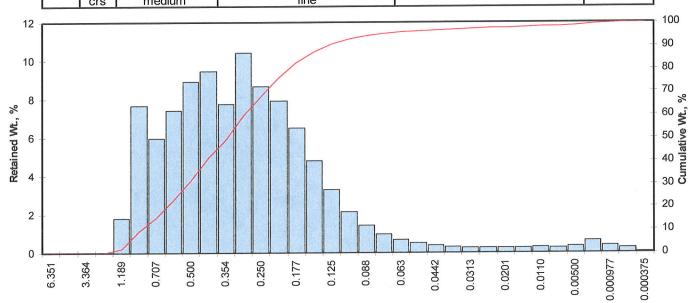
Particle Size Analysis - ASTM D4464M

Client: Project: Project No: Shaw Environmental Port of Tacoma 111487 01000000 PTS File No: Sample ID: 38090

Depth, ft:

SC-1-10.5'-13.0' 12.2





Particle Size, mm

				Sample	Increment	Cumulative
Оре	ening	Phi of	U.S.	Weight,	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	4	0.00	0.00	0.00
0.1324	3.364	-1.75	6	0.00	0.00	0.00
0.0787	2.000	-1.00	10	0.00	0.00	0.00
0.0468	1.189	-0.25	16	1.79	1.79	1.79
0.0331	0.841	0.25	20	7.65	7.65	9.44
0.0278	0.707	0.50	25	5.96	5.96	15.41
0.0234	0.595	0.75	30	7.43	7.43	22.84
0.0197	0.500	1.00	35	8.92	8.92	31.76
0.0166	0.420	1.25	40	9.45	9.45	41.22
0.0139	0.354	1.50	45	7.76	7.76	48.98
0.0117	0.297	1.75	50	10.40	10.40	59.39
0.0098	0.250	2.00	60	8.66	8.66	68.05
0.0083	0.210	2.25	70	7.92	7.92	75.97
0.0070	0.177	2.50	80	6.51	6.51	82.49
0.0059	0.149	2.75	100	4.81	4.81	87.30
0.0049	0.125	3.00	120	3.29	3.29	90.59
0.0041	0.105	3.25	140	2.13	2.13	92.72
0.0035	0.088	3.50	170	1.40	1.40	94.12
0.0029	0.074	3.75	200	0.96	0.96	95.08
0.0025	0.063	4.00	230	0.67	0.67	95.75
0.0021	0.053	4.25	270	0.48	0.48	96.23
0.00174	0.0442	4.50	325	0.37	0.37	96.60
0.00146	0.0372	4.75	400	0.30	0.30	96.90
0.00123	0.0313	5.00	450	0.25	0.25	97.15
0.000986	0.0250	5.32	500	0.27	0.27	97.42
0.000790	0.0201	5.64	635	0.24	0.24	97.66
0.000615	0.0156	6.00		0.23	0.23	97.89
0.000435	0.0110	6.50		0.28	0.28	98.17
0.000308	0.00781	7.00		0.26	0.26	98.43
0.000197	0.00500	7.65		0.32	0.32	98.75
0.000077	0.00195	9.00		0.61	0.61	99.36
0.000038	0.000977	10.00		0.36	0.36	99.72
0.000019	0.000488	11.00		0.25	0.25	99.97
0.000015	0.000375	11.38		0.03	0.03	100.00
TOTALS				100.00	100.00	100.00

Cumulative Weight Percent greater than				
Weight	Phi	Particle Size		
percent	Value	Inches	Millimeters	
5	-0.04	0.0405	1.028	
10	0.27	0.0326	0.827	
16	0.52	0.0275	0.697	
25	0.81	0.0224	0.570	
40	1.22	0.0169	0.430	
50	1.52	0.0137	0.348	
60	1.77	0.0116	0.294	
75	2.22	0.0085	0.215	
84	2.58	0.0066	0.167	
90	2.96	0.0051	0.129	
95	3.73	0.0030	0.075	

Measure	Trask	Inman	Folk-Ward
Median, phi	1.52	1.52	1.52
Median, in.	0.0137	0.0137	0.0137
Median, mm	0.348	0.348	0.348
Mean, phi Mean, in.	1.35 0.0155	1.55 0.0135	1.54 0.0135
Mean, mm	0.392	0.342	0.344
Sorting	1.629	1.029	1.086
Skewness	1.007	0.024	0.097
Kurtosis	0.254	0.831	1.097
Crain Size D	occrintion	ľ	Fine sand

(ASTM-USCS Scale)	(based on Mea	n from Trask)
Description	Retained	Weight

Description	Retained	Weight
	on Sieve#	Percent
Gravel	4	0.00
Coarse Sand	10	0.00
Medium Sand	40	41.22
Fine Sand	200	53.86
Silt	>0.005 mm	3.67
Clay	<0.005 mm	1.25
	Total	100

Particle Size Analysis - ASTM D422M

Client: Project: **Project No:**

Shaw Environmental Port of Tacoma

111487 01000000

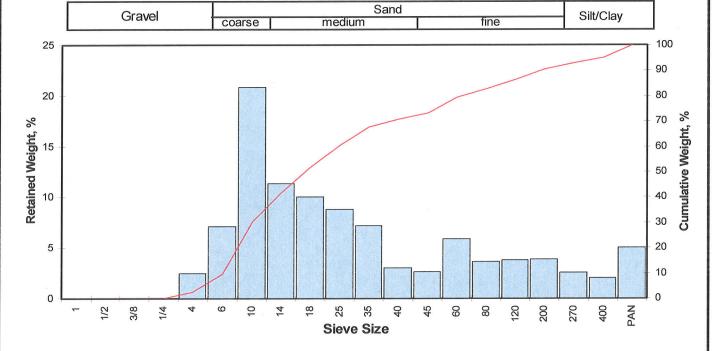
PTS File No: Sample ID:

38090 SC-2-7.5-10.0

Depth, ft:

8.7





			U.S.	Sample	Incremental	Cumulative
Ope	ning	Phi of	Sieve	Weight	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.9844	25.002	-4.64	1	0.00	0.00	0.00
0.4922	12.501	-3.64	1/2	0.00	0.00	0.00
0.3740	9.500	-3.25	3/8	0.00	0.00	0.00
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	4	0.40	2.43	2.43
0.1324	3.364	-1.75	6	1.17	7.12	9.55
0.0787	2.000	-1.00	10	3.42	20.80	30.35
0.0557	1.414	-0.50	. 14	1.86	11.31	41.67
0.0394	1.000	0.00	18	1.65	10.04	51.70
0.0278	0.707	0.50	25	1.44	8.76	60.46
0.0197	0.500	1.00	35	1.18	7.18	67.64
0.0166	0.420	1.25	40	0.50	3.04	70.68
0.0139	0.354	1.50	45	0.43	2.62	73.30
0.0098	0.250	2.00	60	0.97	5.90	79.20
0.0070	0.177	2.50	80	0.59	3.59	82.79
0.0049	0.125	3.00	120	0.62	3.77	86.56
0.0029	0.074	3.75	200	0.63	3.83	90.39
0.0021	0.053	4.25	270	0.42	2.55	92.94
0.0015	0.037	4.75	400	0.33	2.01	94.95
			PAN	0.83	5.05	100.00

Cumulative Weight Percent greater than				
Weight	Phi Particle Size		cle Size	
percent	Value	Inches	Millimeters	
5	-2.07	0.1653	4.198	
10	-1.73	0.1309	3.326	
16	-1.52	0.1127	2.863	
25	-1.19	0.0900	2.286	
40	-0.57	0.0586	1.488	
50	-0.08	0.0418	1.061	
60	0.47	0.0284	0.720	
75	1.64	0.0126	0.320	
84	2.66	0.0062	0.158	
90	3.67	0.0031	0.078	
95	4.70	0.0015	0.038	

I Massaura I	Tunale	T Immon	I Calle Mond
Measure	Trask	Inman	Folk-Ward
Median, phi	-0.08	-0.08	-0.08
Median, in.	0.0418	0.0418	0.0418
Median, mm	1.061	1.061	1.061
Mean, phi	-0.38	0.57	0.35
Mean, in.	0.0513	0.0265	0.0308
Mean, mm	1.303	0.673	0.783
Sorting	2.673	2.089	2.071
Skewness	0.806	0.314	0.364
Kurtosis	0.303	0.621	0.978

Medium sand **Grain Size Description** (ASTM-USCS Scale) (based on Mean from Trask)

Description	Retained on Sieve #	Weight Percent
Gravel	4	2.43
Coarse Sand	10	27.92
Medium Sand	40	40.33
Fine Sand	200	19.71
Silt/Clay	<200	9.61
	Total	100

TOTALS

100.00

16.44

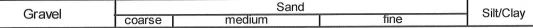
Particle Size Analysis - ASTM D422M

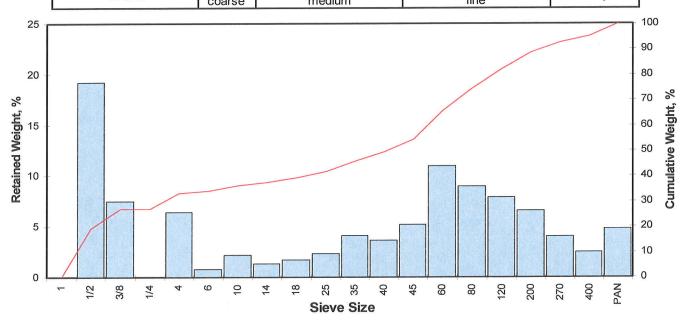
Client: Project: Project No: Shaw Environmental

Port of Tacoma 111487 01000000 PTS File No: Sample ID: 38090 SC-2-10.0-12.5

Depth, ft:

10.7





			U.S.	Sample	Incremental	Cumulative
Ope	ning	Phi of	Sieve	Weight	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.9844	25.002	-4.64	1	0.00	0.00	0.00
0.4922	12.501	-3.64	1/2	3.33	19.23	19.23
0.3740	9.500	-3.25	3/8	1.30	7.51	26.73
0.2500	6.351	-2.67	1/4	0.00	0.00	26.73
0.1873	4.757	-2.25	4	1.11	6.41	33.14
0.1324	3.364	-1.75	6	0.14	0.81	33.95
0.0787	2.000	-1.00	10	0.38	2.19	36.14
0.0557	1.414	-0.50	14	0.23	1.33	37.47
0.0394	1.000	0.00	18	0.30	1.73	39.20
0.0278	0.707	0.50	25	0.40	2.31	41.51
0.0197	0.500	1.00	35	0.71	4.10	45.61
0.0166	0.420	1.25	40	0.63	3.64	49.25
0.0139	0.354	1.50	45	0.89	5.14	54.39
0.0098	0.250	2.00	60	1.90	10.97	65.36
0.0070	0.177	2.50	80	1.55	8.95	74.31
0.0049	0.125	3.00	120	1.36	7.85	82.16
0.0029	0.074	3.75	200	1.13	6.52	88.68
0.0021	0.053	4.25	270	0.70	4.04	92.73
0.0015	0.037	4.75	400	0.43	2.48	95.21
			PAN	0.83	4.79	100.00

Cumulative Weight Percent greater than				
Weight	Phi	Particle Size		
percent	Value	Inches	Millimeters	
5	-4.38	0.8220	20.878	
10	-4.12	0.6864	17.435	
16	-3.81	0.5529	14.043	
25	-2.89	0.2923	7.425	
40	0.17	0.0349	0.887	
50	1.29	0.0161	0.410	
60	1.76	0.0117	0.296	
75	2.54	0.0068	0.171	
84	3.21	0.0042	0.108	
90	3.91	0.0026	0.066	
95	4.71	0.0015	0.038	

Measure	Trask	Inman	Folk-Ward
THE RESERVE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN	THE RESERVE OF THE PERSON NAMED IN COLUMN		THE RESERVE OF THE PERSON NAMED IN
Median, phi	1.29	1.29	1.29
Median, in.	0.0161	0.0161	0.0161
Median, mm	0.410	0.410	0.410
Mean, phi	-1.93	-0.30	0.23
Mean, in.	0.1495	0.0485	0.0336
Mean, mm	3.798	1.231	0.853
Sorting	6.581	3.512	3.133
Skewness	2.752	-0.452	-0.350
Kurtosis	0.209	0.295	0.685

Grain Size Description Coarse sand (ASTM-USCS Scale) (based on Mean from Trask)

Description	Retained on Sieve #	Weight Percent
Gravel	4	33.14
Coarse Sand	10	3.00
Medium Sand	40	13.11
Fine Sand	200	39.43
Silt/Clay	<200	11.32
	Total	100

TOTALS

17.32

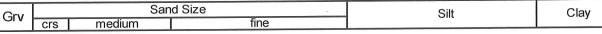
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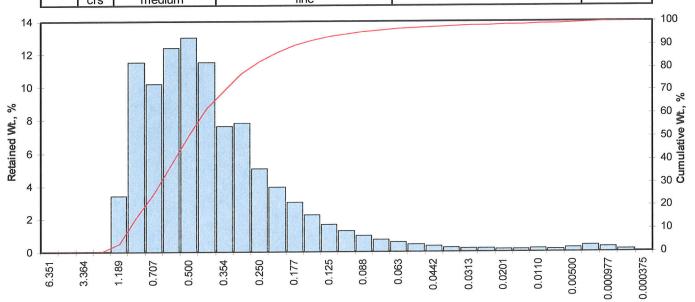
Particle Size Analysis - ASTM D4464M

Client: Shaw Environmental
Project: Port of Tacoma
Project No: 111487 01000000

PTS File No: Sample ID: 38090 SC-2-15.0'-17.5'

Depth, ft: 16.1





Particle Size, mm

				Sample	Increment	
	ening	Phi of	U.S.	Weight,	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	4	0.00	0.00	0.00
0.1324	3.364	-1.75	6	0.00	0.00	0.00
0.0787	2.000	-1.00	10	0.00	0.00	0.00
0.0468	1.189	-0.25	16	3.41	3.41	3.41
0.0331	0.841	0.25	20	11.50	11.50	14.92
0.0278	0.707	0.50	25	10.20	10.20	25.12
0.0234	0.595	0.75	30	12.40	12.40	37.53
0.0197	0.500	1.00	35	13.00	13.01	50.53
0.0166	0.420	1.25	40	11.50	11.50	62.03
0.0139	0.354	1.50	45	7.63	7.63	69.67
0.0117	0.297	1.75	50	7.84	7.84	77.51
0.0098	0.250	2.00	60	5.03	5.03	82.54
0.0083	0.210	2.25	70	3.92	3.92	86.46
0.0070	0.177	2.50	80	2.99	2.99	89.46
0.0059	0.149	2.75	100	2.23	2.23	91.69
0.0049	0.125	3.00	120	1.66	1.66	93.35
0.0041	0.105	3.25	140	1.26	1.26	94.61
0.0035	0.088	3.50	170	0.99	0.99	95.60
0.0029	0.074	3.75	200	0.74	0.74	96.34
0.0025	0.063	4.00	230	0.57	0.57	96.91
0.0021	0.053	4.25	270	0.45	0.45	97.36
0.00174	0.0442	4.50	325	0.32	0.32	97.68
0.00146	0.0372	4.75	400	0.24	0.24	97.92
0.00123	0.0313	5.00	450	0.19	0.19	98.11
0.000986	0.0250	5.32	500	0.20	0.20	98.31
0.000790	0.0201	5.64	635	0.16	0.16	98.47
0.000615	0.0156	6.00		0.15	0.15	98.62
0.000435	0.0110	6.50		0.18	0.18	98.80
0.000308	0.00781	7.00		0.17	0.17	98.97
0.000197	0.00500	7.65		0.22	0.22	99.19
0.000077	0.00195	9.00		0.40	0.40	99.59
0.000038	0.000977	10.00		0.28	0.28	99.87
0.000019	0.000488	11.00		0.13	0.13	100.00
0.000015	0.000375	11.38		0.00	0.00	100.00
TOTALS				100.00	100.00	100.00

Cumulative Weight Percent greater than				
Weight	Phi	Parti	cle Size	
percent	Value	Inches	Millimeters	
5	-0.18	0.0446	1.134	
10	0.04	0.0384	0.975	
16	0.28	0.0325	0.826	
25	0.50	0.0279	0.709	
40	0.80	0.0227	0.575	
50	0.99	0.0198	0.504	
60	1.21	0.0171	0.434	
75	1.67	0.0124	0.314	
84	2.09	0.0092	0.234	
90	2.56	0.0067	0.169	
95	3.35	0.0039	0.098	

Measure	Trask	Inman	Folk-Ward
Median, phi	0.99	0.99	0.99
Median, in.	0.0198	0.0198	0.0198
Median, mm	0.504	0.504	0.504
Mean, phi	0.97	1.18	1.12
Mean, in.	0.0201	0.0173	0.0181
Mean, mm	0.511	0.440	0.460
Sorting	1.502	0.908	0.989
Skewness	0.937	0.215	0.276
Kurtosis	0.245	0.943	1.233

Grain Size Description	Medium sand
(ASTM-USCS Scale)	(based on Mean from Trask)

Description	Retained on Sieve #	Weight Percent
Gravel	4	0.00
Coarse Sand	10	0.00
Medium Sand	40	62.03
Fine Sand	200	34.30
Silt	>0.005 mm	2.85
Clay	<0.005 mm	0.81
	Total	100

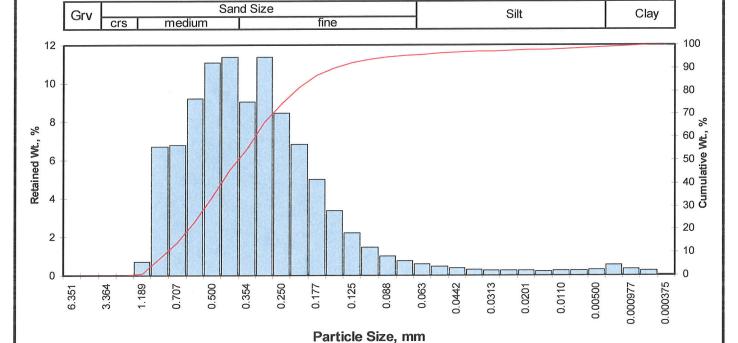
Particle Size Analysis - ASTM D4464M

Client: Shaw Environmental Project: Port of Tacoma Project No: 111487 01000000

PTS File No: Sample ID: Depth, ft:

38090 SC-2-15.0'-17.5'

16.6



				Sample	Increment	Cumulative
Оре	ening	Phi of	U.S.	Weight,	Weight,	Weight,
Inches	Millimeters	Screen	No.	grams	percent	percent
0.2500	6.351	-2.67	1/4	0.00	0.00	0.00
0.1873	4.757	-2.25	4	0.00	0.00	0.00
0.1324	3.364	-1.75	6	0.00	0.00	0.00
0.0787	2.000	-1.00	10	0.00	0.00	0.00
0.0468	1.189	-0.25	16	0.72	0.72	0.72
0.0331	0.841	0.25	20	6.73	6.73	7.45
0.0278	0.707	0.50	25	6.78	6.78	14.22
0.0234	0.595	0.75	30	9.21	9.20	23.42
0.0197	0.500	1.00	35	11.10	11.09	34.52
0.0166	0.420	1.25	40	11.40	11.39	45.91
0.0139	0.354	1.50	45	9.04	9.03	54.94
0.0117	0.297	1.75	50	11.40	11.39	66.34
0.0098	0.250	2.00	60	8.47	8.46	74.80
0.0083	0.210	2.25	70	6.85	6.85	81.65
0.0070	0.177	2.50	80	5.00	5.00	86.64
0.0059	0.149	2.75	100	3.36	3.36	90.00
0.0049	0.125	3.00	120	2.22	2.22	92.22
0.0041	0.105	3.25	140	1.47	1.47	93.69
0.0035	0.088	3.50	170	1.00	1.00	94.69
0.0029	0.074	3.75	200	0.73	0.73	95.42
0.0025	0.063	4.00	230	0.57	0.57	95.99
0.0021	0.053	4.25	270	0.47	0.47	96.46
0.00174	0.0442	4.50	325	0.36	0.36	96.82
0.00146	0.0372	4.75	400	0.29	0.29	97.11
0.00123	0.0313	5.00	450	0.24	0.24	97.35
0.000986	0.0250	5.32	500	0.26	0.26	97.61
0.000790	0.0201	5.64	635	0.23	0.23	97.84
0.000615	0.0156	6.00		0.21	0.21	98.05
0.000435	0.0110	6.50		0.26	0.26	98.31
0.000308	0.00781	7.00		0.24	0.24	98.55
0.000197	0.00500	7.65		0.30	0.30	98.85
0.000077	0.00195	9.00		0.56	0.56	99.41
0.000038	0.000977	10.00		0.34	0.34	99.75
0.000019	0.000488	11.00		0.23	0.23	99.98
0.000015	0.000375	11.38		0.03	0.02	100.00
TOTALS				100.10	100.00	100.00

Cumula	tive Weight F	Percent grea	ater than
Weight	Phi	Parti	cle Size
percent	Value	Inches	Millimeters
5	0.07	0.0376	0.954
10	0.34	0.0310	0.788
16	0.55	0.0269	0.684
25	0.79	0.0228	0.580
40	1.12	0.0181	0.460
50	1.36	0.0153	0.389
60	1.61	0.0129	0.327
75	2.01	0.0098	0.249
84	2.37	0.0076	0.194
90	2.75	0.0059	0.149
95	3.61	0.0032	0.082

Measure	Trask	Inman	Folk-Ward
Median, phi	1.36	1.36	1.36
Median, in.	0.0153	0.0153	0.0153
Median, mm	0.389	0.389	0.389
l			
Mean, phi	1.27	1.46	1.43
Mean, in.	0.0163	0.0143	0.0146
Mean, mm	0.414	0.364	0.372
Sorting	1.527	0.910	0.991
Skewness	0.977	0.104	0.186
Kurtosis	0.259	0.945	1.187
Grain Size D	escription		Fine sand
(ASTM-US	CS Scale)	(based on M	ean from Trask)

Description	Retained	Weight
	on Sieve#	Percent
Gravel	4	0.00
Coarse Sand	10	0.00
Medium Sand	40	45.91
Fine Sand	200	49.51
Silt	>0.005 mm	3.43
Clay	<0.005 mm	1.15
	Total	100

Appendix E Soil Core Drilling Photographs



Photo 1. Drilling boring SC-1.



Photo 3. Core sampler damaged by subsurface rocks.

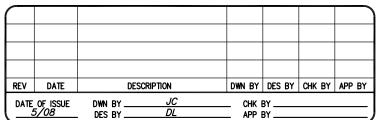




Photo. 2. New Osterberg soil coring device (top) and sampler damaged by subsurface rocks.



Photo 4. Soil filled core sampler prior to capping for shipment to the laboratory.



SOIL CORE DRILLING PHOTOGRAPHS

PORT OF TACOMA FORMER MILWAUKEE RAILYARD SITE TACOMA, WASHINGTON

PROJECT NO. 111487

Appendix F

Torkelson Geochemistry, Inc. NAPL and Groundwater Test Data



Torkelson Geochemistry, Inc.

2528 South Columbia Place, Tulsa, Oklahoma 74114-3233 Voice 918-749-8441. Fax 918-749-6005

May 19, 2008

Larry Roberts
TechSolv Consulting Group, Inc.
7518 NE 169th Street
Kenmore, WA 98028

Subject: Analysis and evaluation of five product and two water samples from Port of Tacoma, Former Millwaukee Rail Yard, Tacoma, Washington.

Introduction

Five product/water sample pairs were submitted to Torkelson Geochemistry by TechSolv for hydrocarbon fingerprint (capillary gas chromatography) analysis of the five product samples and one of the water samples, physical property analysis of two of the product/water pairs and evaluation of the results, see chain of custody (Figure 1).

The following are my interpretations of the data. Please keep in mind that these interpretations are made without any specific knowledge of the site, location from which the samples came, or other analyses done on the samples. In addition, the samples have probably been weathered which makes an accurate interpretation of product type somewhat more difficult since some of the key features of the product have been altered or removed by the evaporation, water washing or bacterial processes.

Discussion of Results

Sample CW-4 appears to be a mixture of extremely weathered middle distillate and a heavier material. The chromatogram of CW-4 (Figures 2 and 11) shows a series of peaks starting at about methylcyclohexane (MCHX) and continuing to the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC17 and continues to about nC24 and a second unresolved hump that starts at about nC24 reaches a maximum at about nC36 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. CW-4 is quite similar to CW-5 and RW-5L3.

Sample CW-5 appears to be a mixture of extremely weathered middle distillate and a heavier material. The chromatogram of CW-5 (Figures 3 and 12) shows a series of peaks starting at about methylcyclohexane (MCHX) and continuing to the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC17 and continues to about nC24 and a second unresolved hump that starts at about nC24 reaches a maximum at about nC36 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. CW-5 is quite similar to CW-4 and RW-5L3.

Sample RW-2L3 appears to be a mixture of extremely weathered middle distillate and perhaps a very small amount of heavier material. The chromatogram of RW-2L3 (Figures 4 and 13) shows a series of peaks starting at about normal octane (nC8) and continuing to near the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC15 or nC16 and continues to about nC24 and a small amount of unresolved material that starts at about nC24 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. RW-2L3 is quite similar to RW-2L5 and fairly similar to RW-5L3. The results of the physical property analyses are given in Table 1.

Sample RW-2L5 appears to be a mixture of extremely weathered middle distillate and perhaps a very small amount of heavier material. The chromatogram of RW-2L5 (Figures 5 and 14) shows a series of peaks starting at about normal nonane (nC9) and continuing to near the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC15 or nC16 and continues to about nC24 and a small amount of unresolved material that starts at about nC24 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. RW-2L5 is quite similar to RW-2L3 and fairly similar to RW-5L3.

Sample RW-5L3 appears to be a mixture of extremely weathered middle distillate and a heavier material. The chromatogram of RW-5L3 (Figures 6 and 15) shows a series of peaks starting at about methylcyclohexane (MCHX) and continuing to the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC17 and continues to about nC24 and a second unresolved hump that starts at about nC24 reaches a maximum at about nC36 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. RW-5L3 is quite similar to CW-4 and CW-5. The results of the physical property analyses are given in Table 1.

The material extracted from water sample RW-5L3 appears to be a mixture of extremely weathered middle distillate and perhaps a small amount of heavier material. The chromatogram of RW-5L3 extract (Figures 7 and 16) shows a series of peaks starting at about normal nonane (nC9) and continuing to near the end of the chromatogram and one unresolved hump that starts at about nC10, reaches a maximum at about nC16 and continues to about nC24 and a small amount of unresolved material that starts at about nC24 and continues to the end of the chromatogram. This includes the ranges typical of middle distillate from about nC8 to nC24 and heavier materials beyond the middle distillate range. The types and proportions of the peaks in the middle distillate range are typical of an extremely weathered diesel fuel or fuel oil. Weathering is primarily biodegradation which has preferentially decreased the size of the normal paraffin peaks (labeled nC8 to nC25) relative to the more biodegradation resistant compounds such as the isoprenoid peaks (labeled IP13 to IP18, Pristane and Phytane). The heavier material beyond the middle distillate range could be a heavy fuel oil, perhaps a lubricating oil or some other heavy material. RW-5L3 is fairly similar to RW-2L3 and RW-2L5.

Please let me know if you have any questions regarding this interpretation.

Bruce Torkelson

Bune lobelson



Torkelson Geochemistry, Inc.

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Page / of /

2528 S. Columbia Place Tulsa, OK 74114-3233

Report/Bill To: Address:

Phone:

Former Millwaukee Rail Yard

Proj. No.: 111487-03000000 P.O.: Sampled B. Chris Milewski

Tacoma WA

Project: Shaw/Port of Tacoma

Location: Port of Tacoma

e-mail: Fax:

Phone: 918-749-8441 e-mail: BTorkelson@torkelsongeochemistry.com Fax: 918-749-6005

7518 NE 1694 St. Henmore, WA 9802 (425) 402 40 40 77 Gizin Inc. 175. T Techsily Lonsoiting Sknd a Roberts Additional Instructions analy-tical Pease Larry

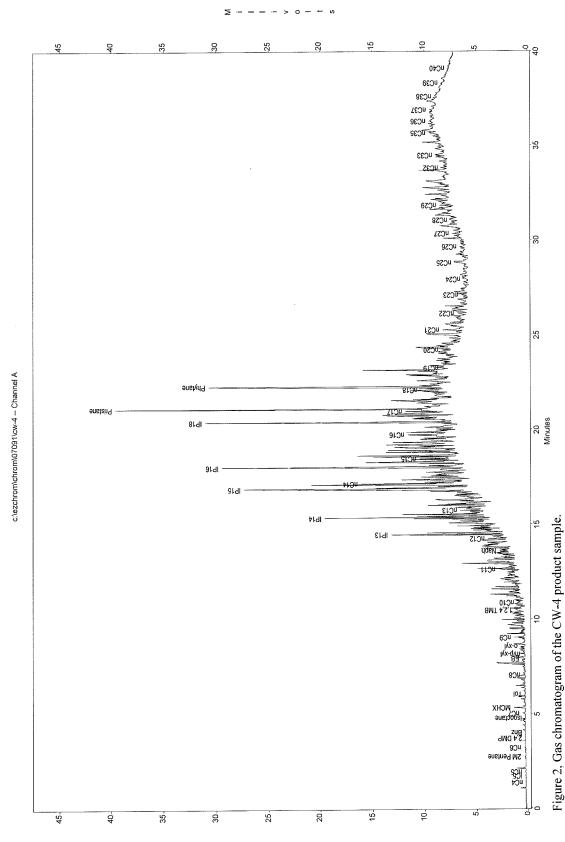
Tobarts (& techsolven con

Standard Requested Turn-Around Time: Shaw Environmental; Brian Peters 425 486 9766 brian peters@shawgrp.com 19909 120th Ave NE Ste 101 Bothell, WA 98011 425 402 3205

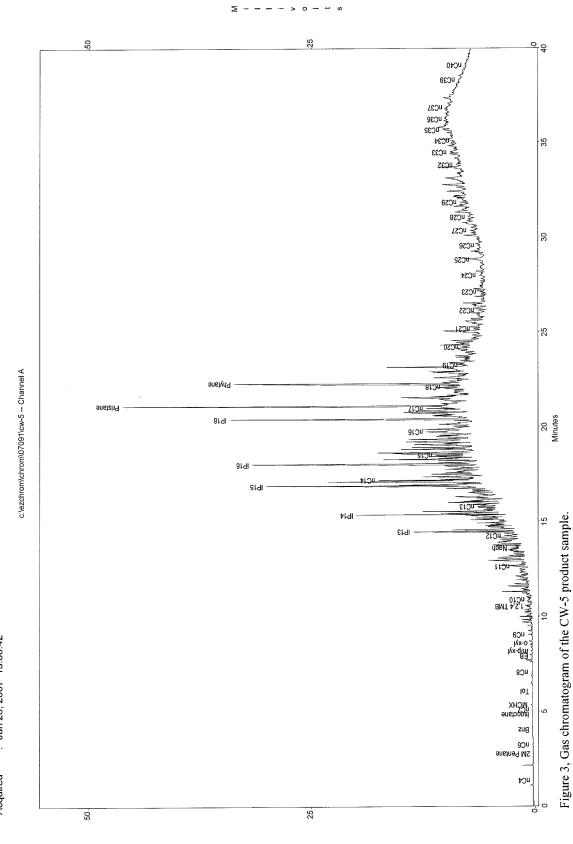
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PRESERVATIVES ANALYSES REQUESTED	Total # OF Visits None GC Characterization Density Viscosity Water Surface Tension MAPL Surface Tension Lead Suffur	3 x	S_{\times}	3×		3 ×	3 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3 x	3 ×	5 x	Z X Z
	MATRIX LABINO.	Probet	Product	Produce	Product	Product	Water	najty.	yaku,	witer	4.c.v
	DATE	6772007	6/8/2007	6/8/2007	6/8/2007	6/8/2007	6/20/2007	6/20/2007	6/20/2007	6/20/2007	6/20/2007
	SAMPLE DESCRIPTION	RW5L3-product	RW2L3-product	RW2L5-product	CW-5-product	CW-4-product	RW2L3-water	RW5L3-water	CW-5-water	CW-4-water	RW-2L5-water
	ITEM NO.	-	2	3	4	5	9	7	8	6	10

RELINQUISHED BY	ACCEPTED BY	DATE	TIME
1000 Jacobson		70/12/00	0,2:51
	Mere Totals	6-22-67	0820

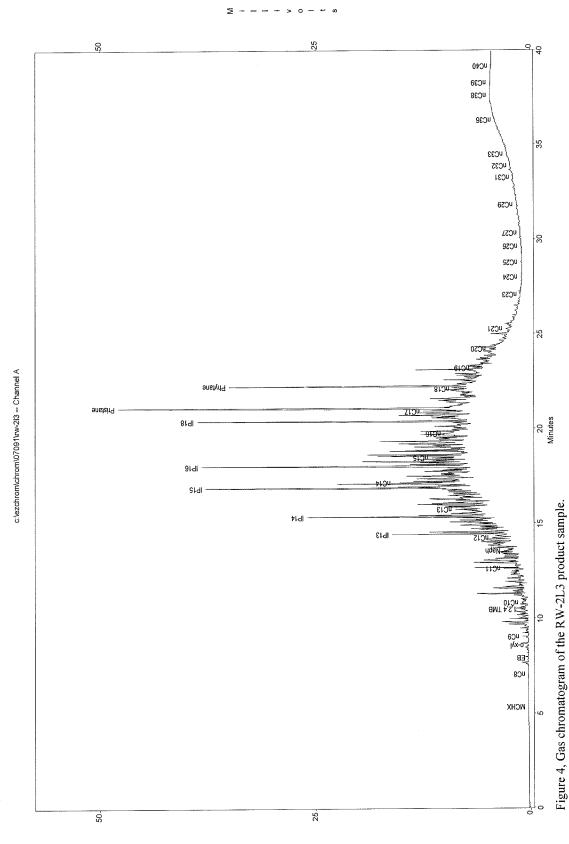
Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : CW4-Product Acquired : Jun 26, 2007 14:04:34



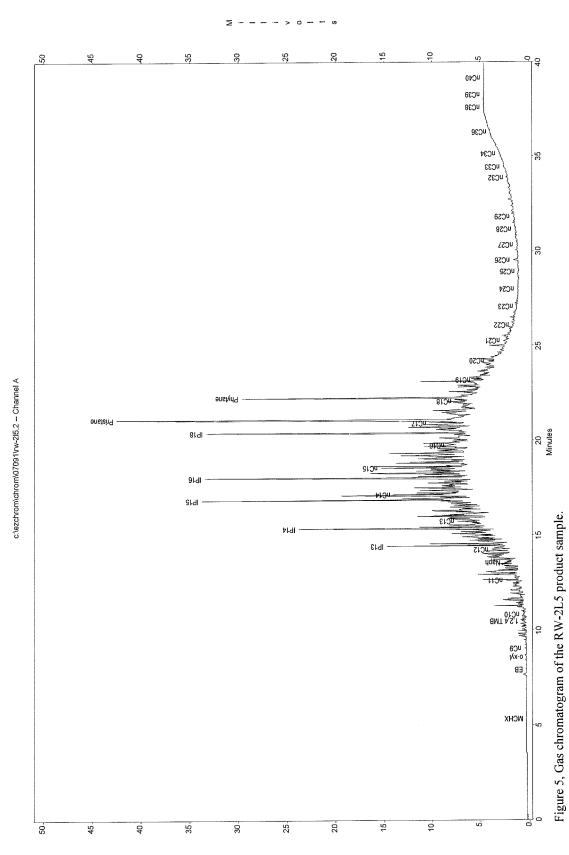
Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : CW-5-Product Acquired : Jun 26, 2007 13:08:42



Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID: RW2L3-Product Acquired: Jun 26, 2007 16:49:41



Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID: RW2L5-Product Acquired: Jun 26, 2007 15:57:58



Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID: RW5L3-Product Acquired: Jun 26, 2007 12:15:29

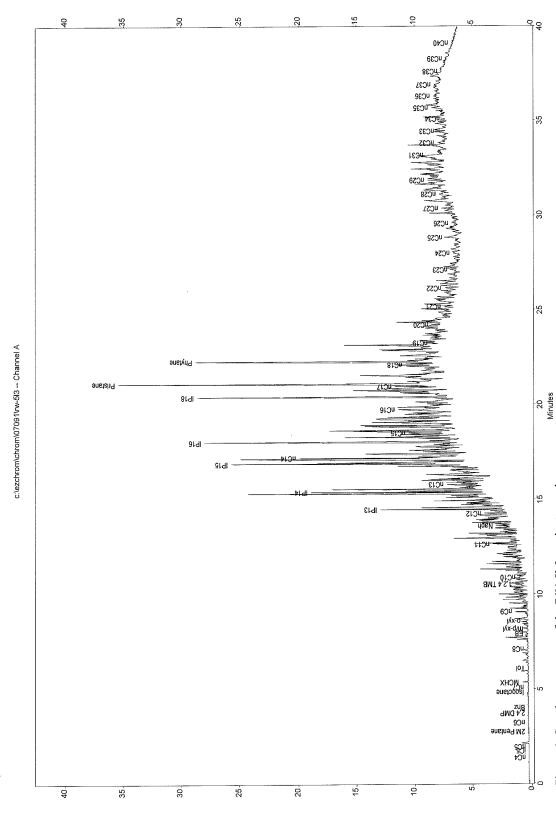
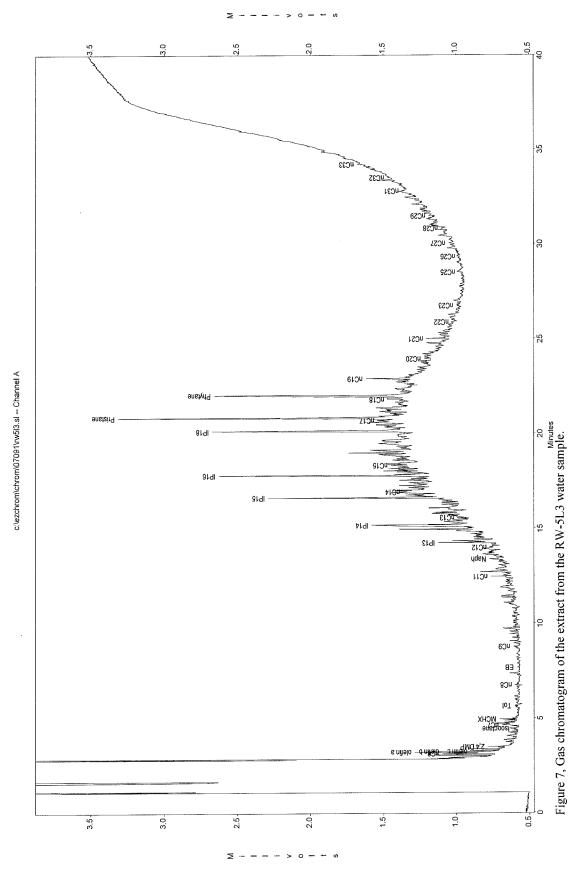
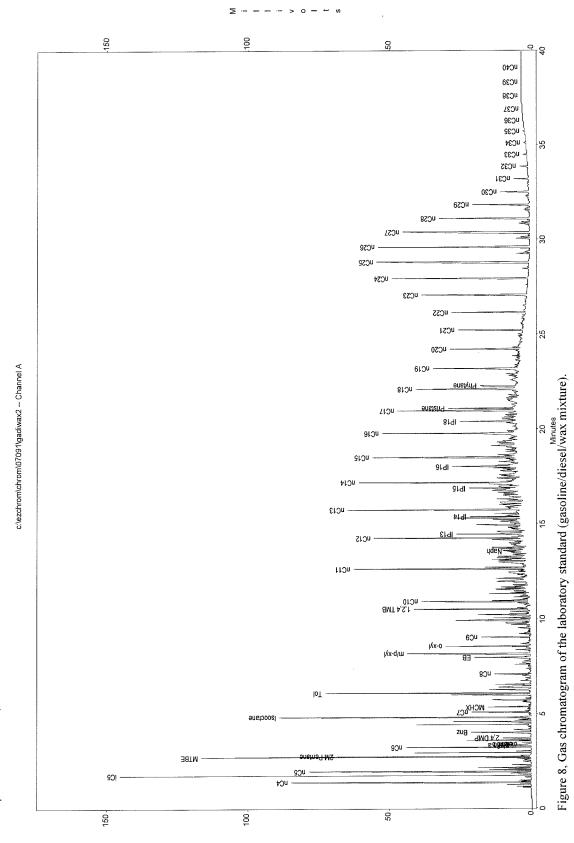


Figure 6, Gas chromatogram of the RW-5L3 product sample.

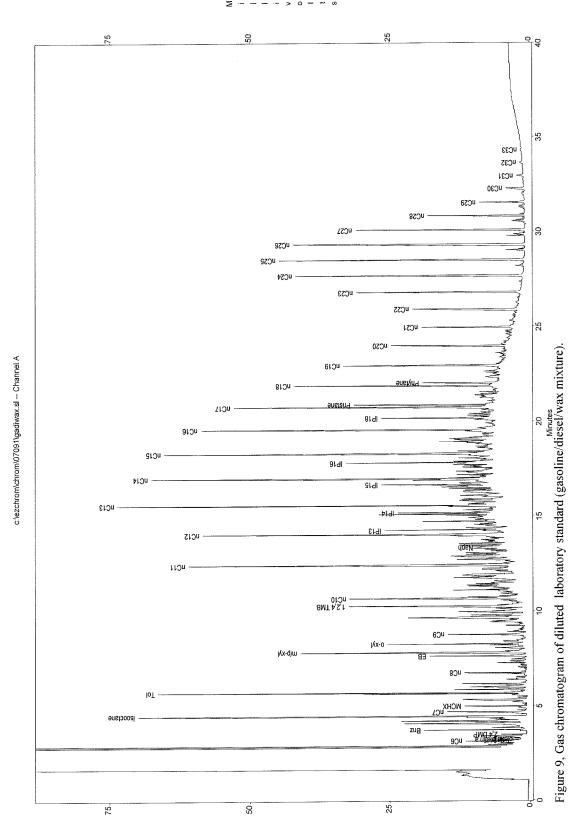
Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : RW5L3 Water Extract Acquired : Jul 11, 2007 13:43:29



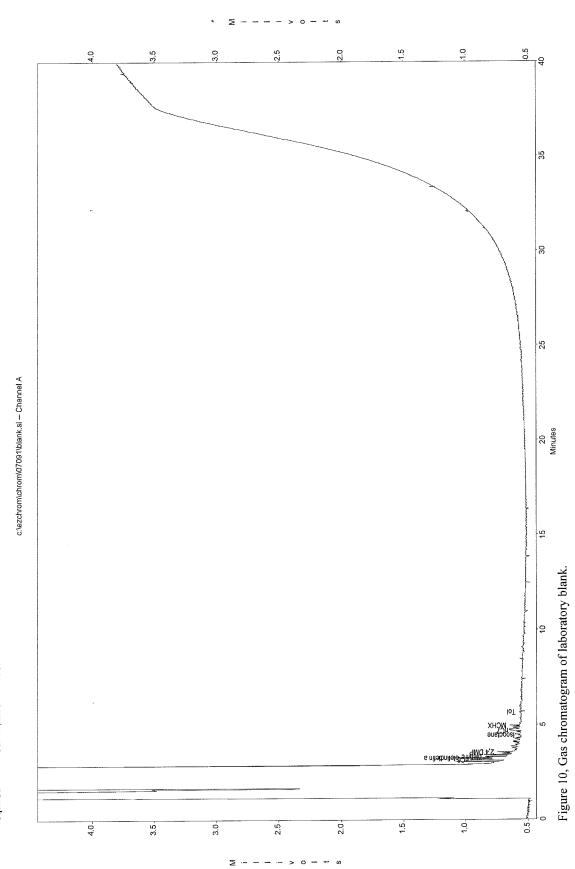
Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : Gas/Dies/Wax std Acquired : Jun 26, 2007 09:44:39



Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : Gas/Dies/Wax std Acquired : Jul 11, 2007 15:23:07



Port of Tacoma, Former Milwaukee Rail Yard, Tacoma, WA Sample ID : Blank Acquired : Jul 11, 2007 14:33:47



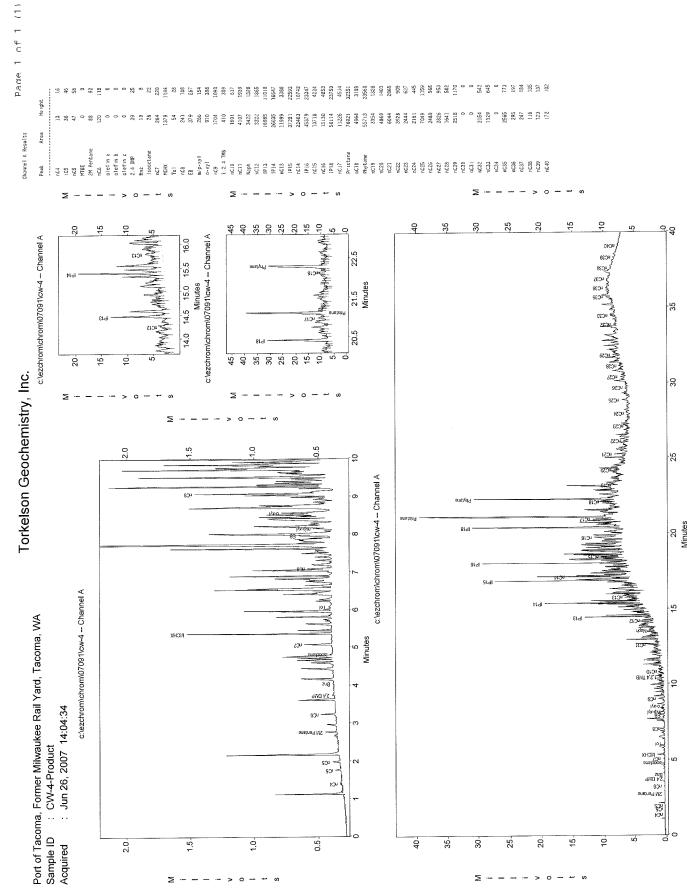
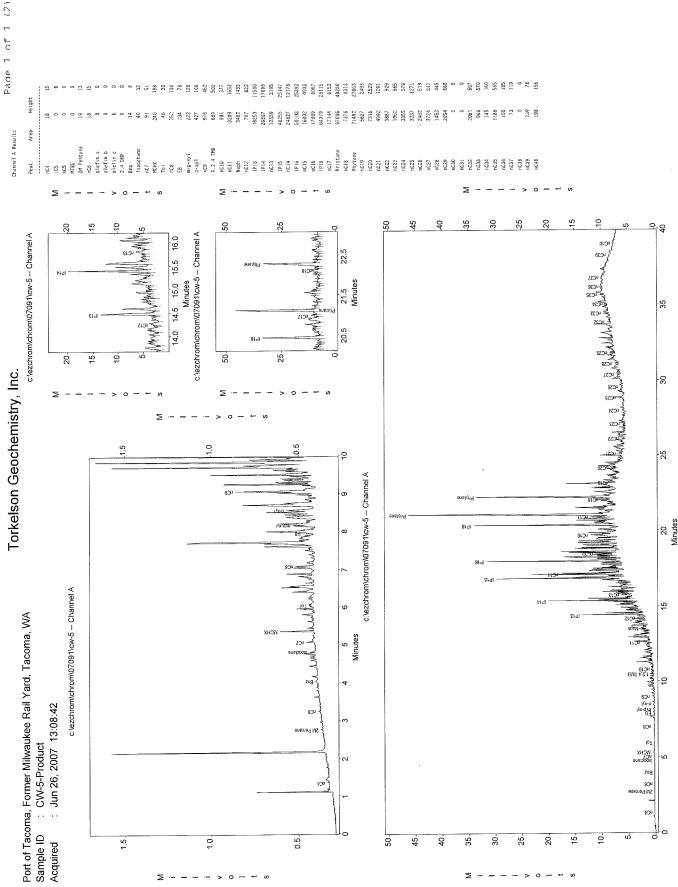


Figure 11, Multipanel display of gas chromatogram of the CW-4 product sample.



 $^{\mbox{\scriptsize Minutes}}$ Figure 12, Multipanel display of gas chromatogram of the CW-5 product sample.

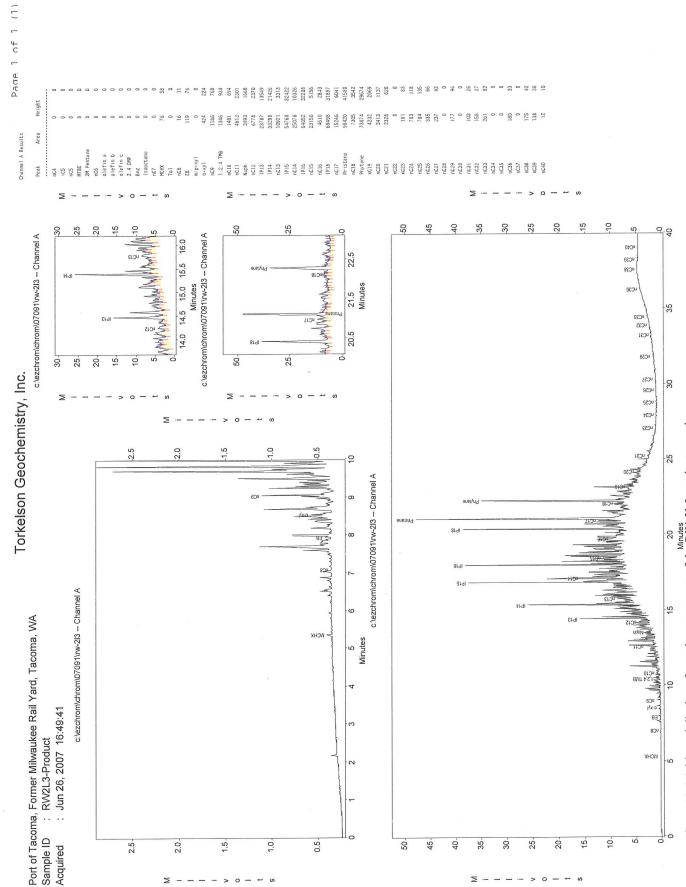


Figure 13, Multipanel display of gas chromatogram of the RW-2L3 product sample.

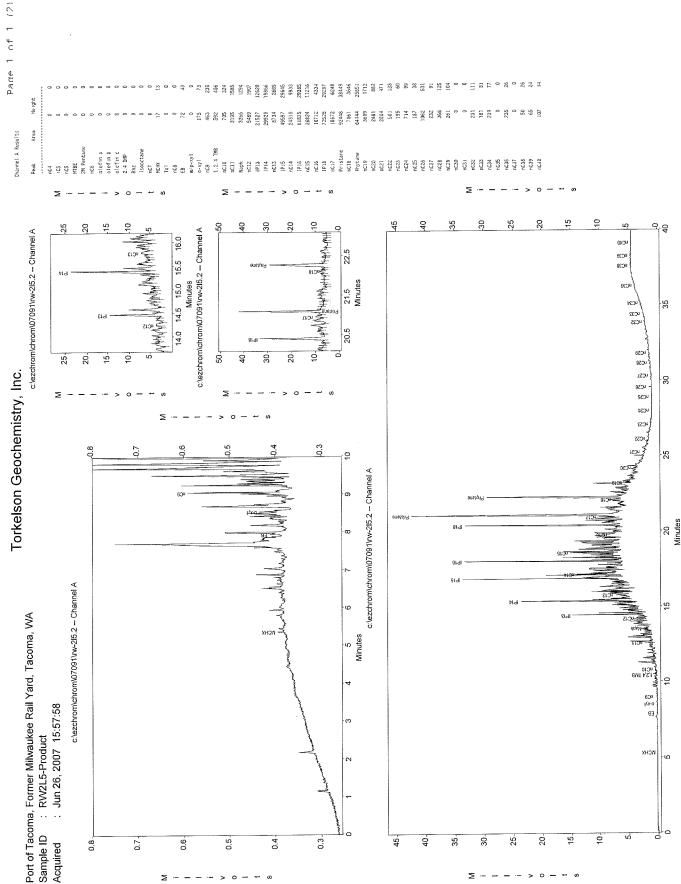
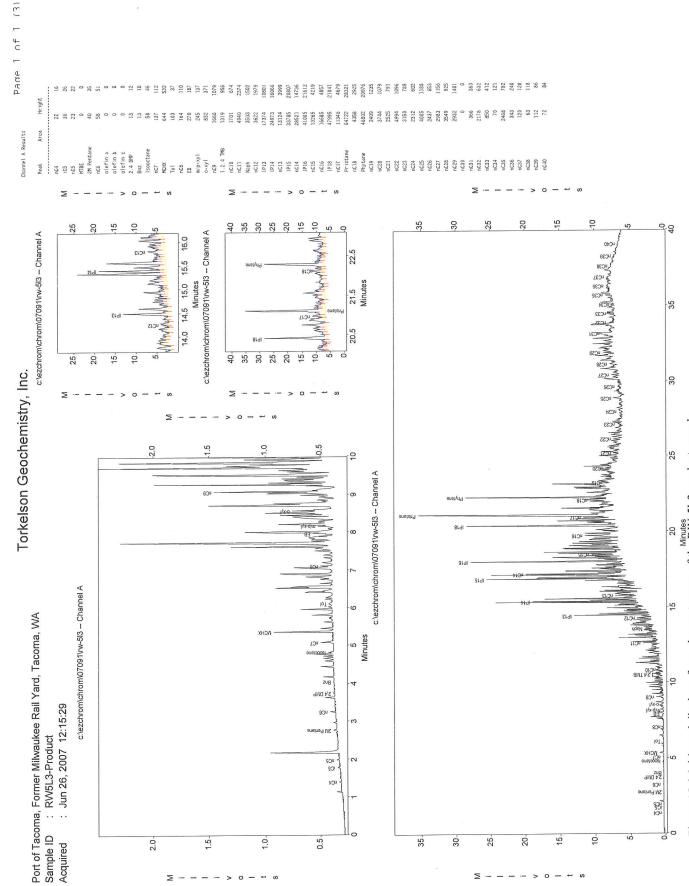
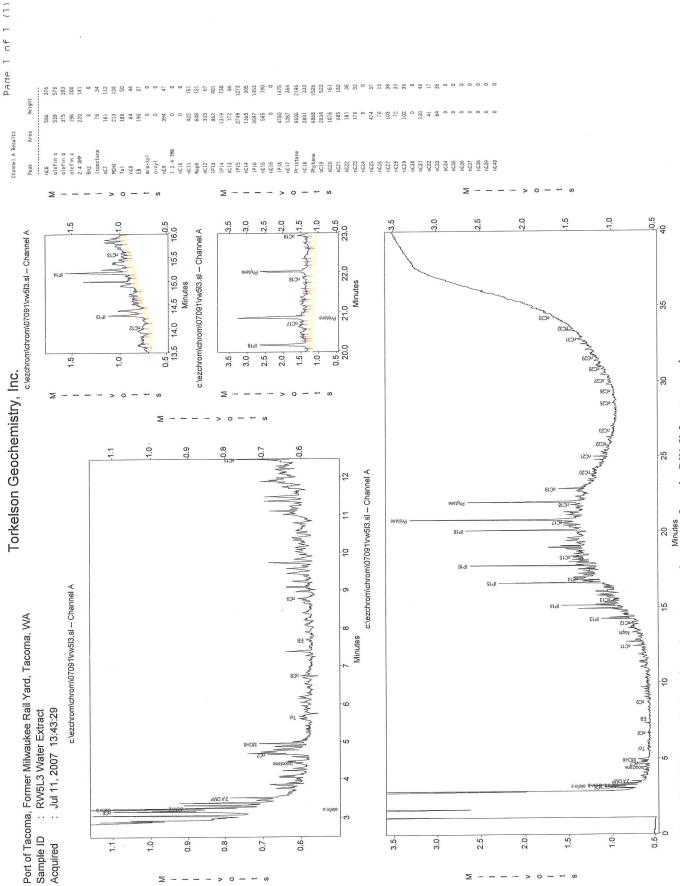


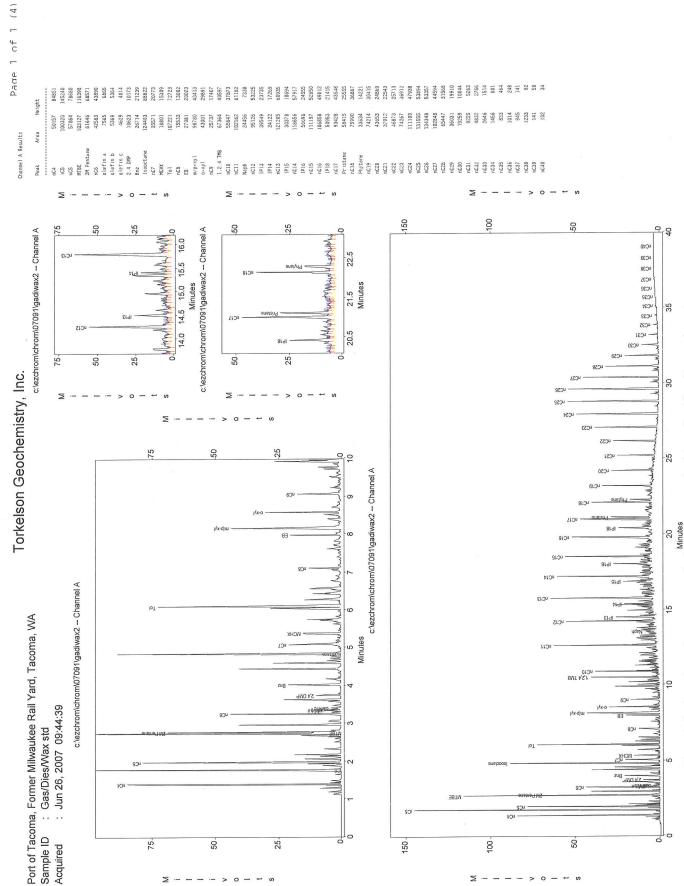
Figure 14, Multipanel display of gas chromatogram of the RW-2L5 product sample.



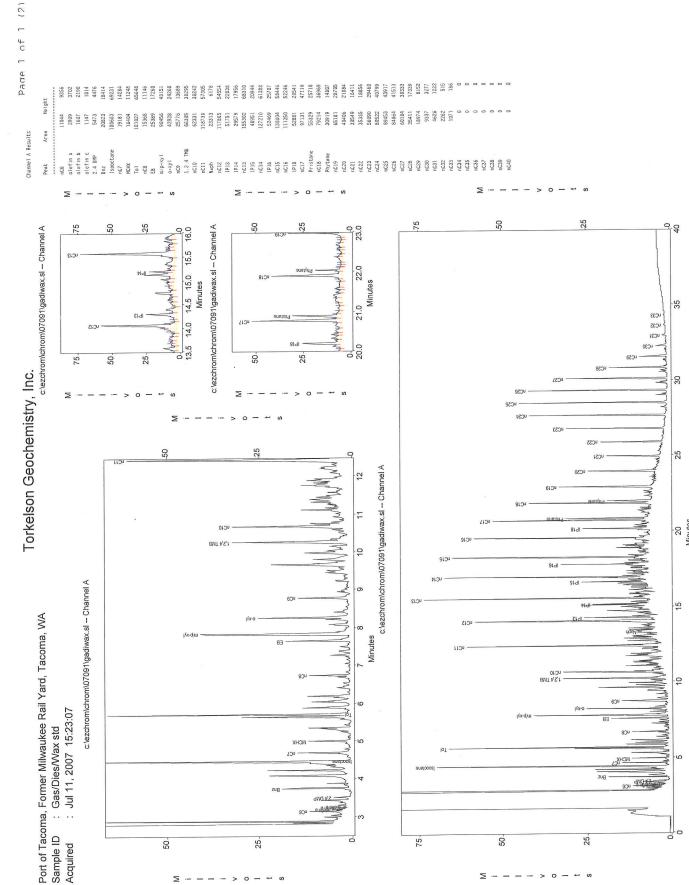
Winutes Figure 15, Multipanel display of gas chromatogram of the RW-5L3 product sample.



Minutes Figure 16, Multipanel display of gas chromatogram the extract from the RW-5L3 water sample.



Winutes Figure 17, Multipanel display of gas chromatogram the laboratory standard (gasoline/diesel/wax mixture).



Pigure 18, Multipanel display of gas chromatogram diluted laboratory standard (gasoline/diesel/wax mixture).

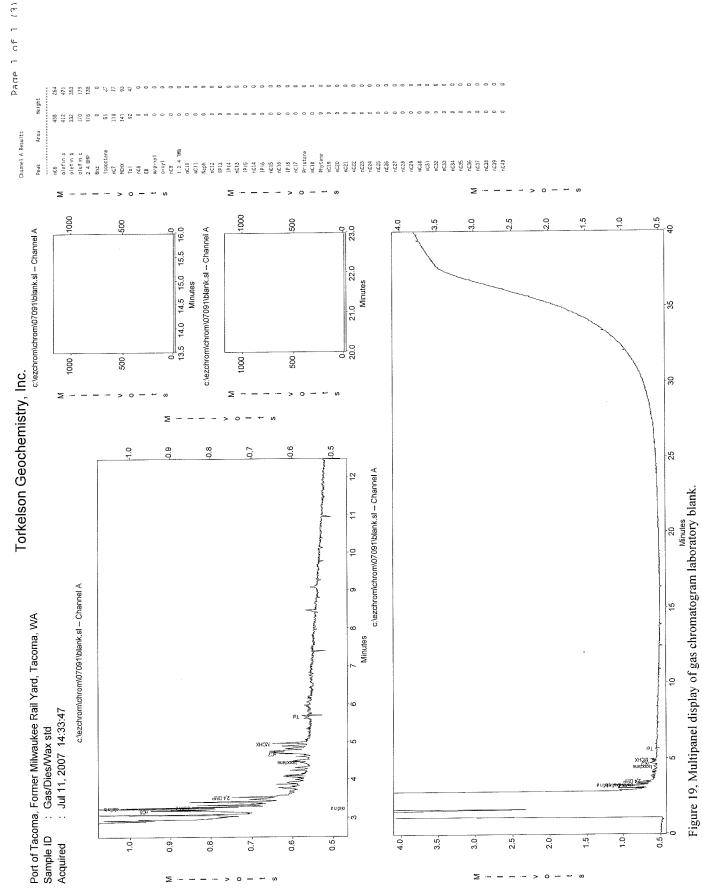


Table 1, Physical Properties

			Torkels	Torkelson Geochemistry, Inc.	ry, Inc.		
		4	hysical P	Physical Properties Measurements	surements		
Sample	TGI Job	Density	Viscosity	Surface Tension	Surface Tension Interfacial Tension Surface Tension	Surface Tension	Temperature of
		(Jm/mb)	(centipoise)	Air/Water	NAPL/Water	Air/NAPL	Measurements
				(dynes/cm)	(dynes/cm)	(dynes/cm)	(Fahrenheit)
RW2L3	07091	0.8803	8.34	58.4	1.7	24.6	09
RW5L3	07091	0.9498	633	56.5	13.7	29.6	9